1	Impact of time pressure on acceleration behavior and crossing decision at
2	the onset of yellow signal
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26 Abstract: This study investigates acceleration behavior and crossing decision of the drivers 27 under increasing time pressure driving conditions. A typical urban route was designed in a 28 fixed-base driving simulator consisting of four signalized intersections with varying time to 29 stop line (4 s and 6 s) and maneuver type (right-turn and go-through). 97 participants' data 30 were obtained under No Time Pressure (NTP), Low Time Pressure (LTP), and High Time 31 Pressure (HTP) driving conditions. The acceleration behavior was examined at the onset of 32 yellow signal in four ways: continuous deceleration, acceleration-deceleration, deceleration-33 acceleration, and continuous acceleration. A random forest model was used to build an 34 acceleration behavior prediction model for identifying the significant explanatory variables 35 based on variable importance ranking. Further, a Mixed Effects Multinomial Logit (MEML) 36 model was developed using the explanatory variables obtained from a random forest model. 37 Additionally, a generalized linear mixed model was incorporated for estimating the likelihood 38 of crossing an intersection by considering all the explanatory variables. A MEML model result 39 revealed that the odds of adopting acceleration-deceleration, deceleration-acceleration, and 40 continuous acceleration instead of continuous deceleration increased by 63 %, 123 %, and 77 41 %, respectively under HTP driving conditions. Moreover, the likelihood of crossing a 42 signalized intersection increased by 2.73 times and 4.26 times when the drivers were under 43 LTP and HTP driving conditions, respectively as compared to NTP driving condition. Apart 44 from this, time to stop line (reference: 6 s) and age showed negative association with crossing 45 probability. Overall, the findings from this study revealed that drivers altered their acceleration 46 behavior for executing risky driving decisions under increasing time pressure driving 47 conditions.

Keywords: Acceleration behavior; Crossing decision; Yellow signal; Time pressure; Random
forest; Mixed effects multinomial logit model.

50 1. Introduction

Intersection is an integral part of roadway system providing access to numerous vehicles intending to converge, diverge, or go through as per their desired destination. Intersection is a common space shared by numerous vehicles at the same time leading to traffic conflicts of varying severity. Drivers need to judge speed and direction of other vehicles to safely cross the intersection area. A minute error in judgement from a driver may lead to road crash (AASHTO, 2011; Mathew, 2009). Due to this reason, traffic signals are installed to minimize hazardous vehicular interactions by providing right-of-way to non-conflicting traffic movements at 58 regular intervals. Installation of traffic signals is found to be an effective solution for reducing traffic crashes occurring at un-signalized intersections (Indo-HCM, 2017; Mathew, 2009). 59 60 However, safety analysis of signalized intersections revealed that drivers deliberately or 61 accidentally either abruptly stop or cross the intersection during the onset of yellow/red signal 62 leading to traffic conflicts. According to the 2018 annual road crash statistics of the USA, 63 nearly 32 % of the total intersection-related crashes befell at signalized intersections (U.S. 64 Department of Transportation Federal Highway Administration, 2021). In China, 30 % of the 65 total traffic crashes occurred in the vicinity of signalized intersections (Jiang et al., 2021). The study conducted by the State of Queensland, Department of Transport and Main Roads (2018) 66 67 revealed that 5.3 % of fatalities ensued at signalized intersections. Great Britain recorded 31.89 68 % KSI (Killed or Seriously Injured) crashes at signalized intersections (Murphy et al., 2020). 69 In 2019, India recorded 7.74 % intersection-related crashes at signalized intersections with 7.14 70 % fatalities. These statistics indicate that driving behavior at signalized intersection must be 71 assessed in terms of driving attributes, driver demographics, driving condition, and other 72 related factors to investigate the factors persuading drivers' decision and compromising their 73 safety.

74 Prior research has documented that decision-making during yellow signal is complex and 75 critical than red signal because drivers need to quickly decide whether to stop or cross the 76 intersection (Choudhary and Velaga, 2019; Haque et al., 2016; Mishra and Zhu, 2015). Yellow 77 signal is a warning of a forthcoming change in the right-of-way. It is safe for the drivers to stop 78 before the intersection after the onset of yellow signal. The drivers approaching a signalized 79 intersection may experience dilemma and fail to take a decision, either to stop or cross when 80 the signal changes from green to yellow (Elmitiny et al., 2010; Eluru and Yasmin, 2016). An 81 inappropriate decision at the onset of yellow signal might result in red light running or abrupt 82 braking to stop at the intersection. Here, it is important to note that drivers' decision can be 83 highly influenced by various driving conditions such as distraction, inattention, time pressure, 84 etc. (Bonneson and Zimmerman, 2004; Palat and Delhomme, 2016). It is observed that the 85 crossing probability decreases during distraction and inattention whereas increases in time 86 pressure as compared to normal driving conditions (Choudhary and Velaga, 2019; Fitzpatrick 87 et al., 2017). Time pressure is defined as a driving condition where drivers are under 88 psychological stress to reach their desired destination within constrained time (Dogan et al., 89 2011; Gelau et al., 2011; Pawar and Velaga, 2020; Rendon-Velez et al., 2016). Due to this 90 reason, drivers adopt high speed driving in order to cover maximum distance in minimum time

91 (Fitzpatrick et al., 2017; Rendon-Velez et al., 2016). Drivers under time pressure at signalized 92 intersections intentionally take risky driving decisions (red light running) to save time which 93 could have been lost while waiting during yellow and red signals. The odds of taking risky 94 decisions varies as per the extent of time constraint and its perception from the drivers during 95 time pressure driving conditions (Cœugnet et al., 2013; Fitzpatrick et al., 2017). However, 96 drivers traveling at high speed fail to gain rapid acceleration within short distance which might 97 alter their crossing decision at signalized intersection (Palat and Delhomme, 2016). Therefore, 98 the current study is conducted to evaluate acceleration behavior of the drivers and its influence 99 on crossing decisions at the onset of yellow signal under time pressure driving conditions.

100 The remainder of the paper is organized as follows. The following section broadly 101 discusses various studies conducted on evaluating driving behavior and crossing decision at 102 signalized intersections. Section 3 provides description related to the design of study. Section 103 4 demonstrates statistical modeling results and its interpretation. Section 5 presents discussion 104 on the results obtained from the current study. Section 6 concludes the paper and section 7 105 highlights the important contributions of the study. Section 8 describes limitations and future 106 scope of the study.

107 2. Previous work on driver behavior at signalized intersection

108 An extensive literature review revealed that abundant research work has been conducted 109 to assess drivers' decision at signalized intersection in real and simulated worlds. The following 110 sub-sections provide an overview of previous research studies considering driving environment 111 (real and simulation), time pressure, acceleration behavior, and crossing decision.

112 2.1 Previous field and experimental studies on signalized intersection

Over the years, traffic safety researchers showed special interest in analyzing driving 113 114 behavior and decision making of the drivers at signalized intersections. Majority of the work 115 was conducted in field where the researchers collected video graphic data for modeling drivers' 116 crossing decisions (Kassim et al., 2014; Kim, W., Zhang, J., Fujiwara, A., Jang, T. Y., & 117 Namgung, 2008; Kumar et al., 2019; Pathivada and Vedagiri, 2021; Rakha et al., 2008; Tarko, 118 Andrew, Wei Li, 2006; Yan et al., 2005). Researchers developed binary logit model for 119 estimating stopping or crossing probability at the onset of yellow signal as a function of speed, 120 distance, perception-reaction time, deceleration rates, category of vehicle, and time of day. In 121 the end, dilemma zone boundaries were proposed to counter the influence of uncertainty at the 122 onset of yellow signal (Papaioannou, 2007; Pathivada and Vedagiri, 2021). In the similar 123 direction, driving simulator experiments were conducted for predicting drivers' decision using 124 binary logit model to develop innovative countermeasures for yellow and red light violations 125 (Hussain et al., 2020b, 2020a). Various driving simulator experiments were conducted to 126 investigate drivers' decision at signalized intersection by considering potential implications of 127 human factors (Abdel-Aty et al., 2009; Caird et al., 2007; Hussain et al., 2020b). Majority of 128 driving simulator studies analyzed crossing decision of the drivers in distracted state 129 (distraction due to mobile phone, eating, drinking, etc.) (Choudhary and Velaga, 2020, 2019; 130 Haque et al., 2016, 2012). The authors adopted hybrid approach of decision tree and logistic 131 regression for modeling drivers' crossing decision at signalized intersections (Ali et al., 2021; Choudhary and Velaga, 2019; Haque et al., 2016). Apart from this, Caird et al. (2007) 132 133 performed a driving simulator study for examining older and younger drivers' driving 134 performance at the onset of yellow signal. The authors observed that older drivers approached 135 intersection at lower speed and most likely stopped at the intersection after the onset of yellow 136 signal. Jahangiri et al. (2016) studied field as well as driving simulator data to predict red light violation at signalized intersections. The authors adopted random forest technique to develop 137 138 red light violation prediction model. It was reported that driving simulator data resulted in more 139 accurate prediction as compared to field data. This was mainly due to the fact that driving 140 simulator data accounted for driver characteristics (age and gender), scenario configurations, 141 and specific driving conditions (usage of handheld and hands-free mobile phone) (Jahangiri et 142 al., 2016).

143 2.2 Factors influencing crossing decision at signalized intersections

144 Extensive research conducted on signalized intersection showed that drivers' crossing 145 decision was mainly influenced by driving speed and distance to stop line at the onset of yellow 146 signal (Papaioannou, 2007; Pathivada and Vedagiri, 2021). It was observed that drivers 147 traveling with speed more than 80 kph and 113 m away from the stop line at the onset of yellow 148 signal were more likely to cross the intersection (Elmitiny et al., 2010). Apart from this, 149 reaction time and mean acceleration had substantial impact on crossing decision of the drivers. 150 The crossing probability was observed to decrease with increment in reaction time after the 151 onset of yellow signal resulting in abrupt deceleration from the drivers (Rakha et al., 2008; 152 Zhang et al., 2014).

153 Crossing decision was also observed to vary according to driver demographics and 154 scenario configuration. Interestingly, research conducted by Haque et al. (2016) and Abdel-155 Aty et al. (2009) revealed contrary findings related to crossing decision of male and female 156 drivers. Haque et al. (2016) reported that female drivers were more likely to cross the 157 intersection whereas Abdel-Aty et al. (2009) stated that female drivers were more likely to stop 158 at the intersection. Further, Choudhary and Velaga (2019) studied the influence of age and type 159 of maneuver on crossing decision at the onset of yellow signal. The authors reported that non-160 distracted mid-age drivers approaching go-through intersection with driving speed of more than 161 57 kph had high chances of crossing the intersection when the signal turned from green to 162 yellow. Ali et al. (2021) examined crossing decision of the drivers in a connected environment 163 and observed that young drivers with driving speed less than 37 kph, variation in longitudinal acceleration more than 0.43 m/s^2 , and driving experience of more than or equal to 8.25 years 164 had lower propensity to cross the intersection at the onset of yellow signal. Caird et al. (2007) 165 166 discovered that 92 % young-age drivers and 75 % old-age drivers stopped at the intersection 167 when time to stop line was 3.58 seconds. Thus, it can be understood that driving attributes 168 along with driver demographics and scenario configuration significantly influenced drivers' 169 crossing decision at the onset of yellow signal.

170 2.3 Effects of time pressure on acceleration behavior and crossing decision

171 Time pressure is one of the foremost contributors to aggressive and risky driving behavior (Cœugnet et al., 2013; Peer, 2010). Numerous studies were conducted in the last 172 173 decade for evaluating driving behavior under time pressure driving conditions (Gelau et al., 174 2011; Lee and LaVoie, 2018; Pawar and Velaga, 2021a; Schmidt-daffy, 2013). However, 175 majority of the research work focused on driving behavior at midblock sections and very 176 limited studies examined drivers' decision making at intersections (Gelau et al., 2011; 177 Paschalidis et al., 2018; Rendon-Velez et al., 2016). Fitzpatrick et al. (2017), Palat and 178 Delhomme (2016), and Dogan et al. (2011) analyzed driving behavior of the drivers at 179 signalized intersections. Fitzpatrick et al. (2017) examined drivers' crossing decision when the 180 signal turned from green to yellow under No Time Pressure (NTP), Low Time Pressure (LTP), 181 and High Time Pressure (HTP) driving conditions. NTP was the baseline condition where no 182 time constraint was imposed whereas LTP and HTP driving conditions demanded drivers to 183 complete the driving sessions in constrained time. The authors requested drivers to complete LTP and HTP driving sessions within 85th and 15th percentile travel time recorded during NTP 184 185 driving session. It was observed that drivers under HTP accelerated faster than LTP and NTP

186 driving conditions. Around 68 % of the total drivers were observed to cross the intersection under HTP driving conditions. No significant increment in crossing decisions was observed in 187 188 LTP and NTP driving conditions (Fitzpatrick et al., 2017). Palat and Delhomme (2016) studied 189 yellow signal and red signal running tendency of the drivers under NTP and time pressure 190 driving conditions. The risk perception analysis revealed that drivers considered traffic signal 191 violation as a risky decision. However, significant number of drivers were observed to violate 192 traffic signal regulations under time pressure (Palat and Delhomme, 2016). Dogan et al. (2011) 193 analyzed driving behavior of the drivers in the vicinity of signalized intersection under time 194 pressure driving conditions. It was observed that drivers under time pressure approached the 195 intersection at high speed, swiftly reacted and hastily decelerated to stop the vehicle (Dogan et 196 al., 2011). Table 1 provides a summary of the previous research work conducted on signalized 197 intersections considering potential implications of human factors.

199 Table 1 Summary of previous research work conducted on driving simulator for evaluating crossing decision at the onset of yellow signal

Study	Sample size	Scenario configuration	Statistical modeling	Major findings
Time pressure stud	lies			
Fitzpatrick et al. (2017)	36	YSD: 4 seconds	Unpaired t-test	Drivers under high time pressure were more likely to cross the intersection
Palat and Delhomme (2016)	94	DSL: 59 m; YSD: 4.3 seconds	Chi-square test	Time pressure increased to likelihood of crossing the intersection when the signal turned from green to yellow
Dogan et al. (2011)	36	TSL: 3 seconds; YSD: 2 seconds	Univariate ANCOVA	Drivers under time pressure approached the intersection at high speed, swiftly reacted and adopted abrupt braking to stop the vehicle
Other relevant stud	lies			
Ali et al. (2021)	78	TSL: 5 seconds; YSD: 3 seconds	Decision tree and panel mixed logit model	Drivers in connected environment showed less propensity to cross the intersection at the onset of yellow signal

Hussain et al. (2020b)	67	DSL: 80 m and 95 m; YSD: 4 seconds	Logistic regression	Unit increase in speed (kph) at the onset of yellow signal increased crossing probability by 5.3 %
Choudhary and Velaga (2019)	74	TSL: 3, 4, and 5 seconds; YSD: 3 seconds	Decision tree and generalized linear mixed model	The chances of crossing an intersection were 17% lesser for the music player distraction than phone conversation
Haque et al. (2016)	69	TSL and YSD: 3 and 3.75 seconds;	Decision tree and generalized estimation equations	Young and mid-aged drivers showed low propensity of crossing whilst distracted irrespective of driving speed
Abdel-Aty et al. (2009)	62	DSL: 90 m; YSD: 4.3 seconds	Logistic regression	High variability in crossing decisions were observed at signalized intersection with high risk of rear-end conflict
Caird et al. (2007)	77	YSD: 4.08, 4.58, and 5.08 seconds	Logistic regression	A perception response time of 1 second was deemed sufficient for all age groups to detect change in signal from green to yellow

200 TSL = Time to Stop Line; YSD = Yellow Signal Duration; DSL = Distance to Stop Line; ANCOVA = Analysis of covariance

201 2.4 Research objective and hypothesis

202 Majority of field studies conducted for evaluating crossing decision at signalized 203 intersection were unable to account the effect of human factors and acceleration behavior. This 204 was mainly because of the limitation of data collection technique. Video-graphic data 205 collection is useful for capturing crossing and stopping decisions of vehicle population at a 206 particular signalized intersection (Pathivada and Vedagiri, 2019). Video-graphic data fails to 207 capture individual driver's driving behavior in terms of accelerator pedal and brake pedal 208 applications and driving conditions (use of mobile phone, alcohol-impaired driving, time 209 pressure, etc.). Further, it is very difficult to capture individual driver's driving behavior during 210 yellow phase in field conditions because there is very limited time for encountering yellow 211 signal and the driver has to precisely approach the signalized intersection at the onset of yellow signal. Due to these limitations, the current study was conducted on a driving simulator with a 212 213 primary goal of investigating the impact of time pressure on acceleration behavior and its 214 influence on drivers' decision (stop/cross) at the onset of yellow signal. Three broad research 215 hypotheses were derived for the current study as shown below:

- (i) Time pressure will alter drivers' acceleration behavior and crossing decisions at theonset of yellow signal.
- (ii) Variation in time to stop line will affect drivers' acceleration behavior and crossing
 decisions at the onset of yellow signal.
- (iii) Driver demographics will influence drivers' acceleration behavior and crossing
 decisions at the onset of yellow signal.
- 222 **3. Materials and Methods**
- 223 3.1 Design of experiment

224 An extensive literature review was conducted to identify the problems or limitations from 225 the previous studies. It was observed that acceleration behavior at the onset of yellow signal 226 under time pressure conditions was not evaluated in the past. Thus, the current study was 227 designed to examine acceleration behavior and crossing decision of the drivers at the onset of 228 yellow signal under increasing time pressure driving conditions. The data related to 229 acceleration behavior and driver decisions at the onset of yellow signal under time pressure 230 driving conditions can be limited, strenuous, difficult, and unsafe to obtain from the field 231 experiment. Due to this reason, a driving simulator study was performed to collect driving 232 behavior data in a controlled environment. Four signalized intersections with varying scenario

configurations were developed on an urban arterial and the data was collected under NTP
(baseline), LTP, and HTP driving conditions. Different statistical analysis techniques
(explained in section 3.6) were considered and modeling of acceleration behavior and crossing
decisions were performed to infer insights from the experiment. Fig. 1 depicts the methodology

237 followed for the current study.

238





240 **3.2** Data collection

241 This research is a part of larger study focused on examining driving behavior of the 242 drivers under time pressure driving conditions. The traffic events most commonly observed on 243 urban arterial roads such as hazardous situations (pedestrians crossing and obstacle overtaking), car-following, un-signalized intersections and signalized intersections were 244 developed in a driving simulator on a 6 km road for investigating the detrimental effects of 245 246 time pressure on driving behavior of the drivers (Pawar et al., 2020; Pawar and Velaga, 2020; 247 Pawar and Velaga, 2021a, 2021b). The further details on data collection and analysis related to 248 hazardous events and car-following event can be found in Pawar et al. (2020) and Pawar and 249 Velaga (2021), respectively. The current study analyzes acceleration behavior and crossing decisions of the drivers at signalized intersections. The following sub-sections present 250 251 description of each component of data collection and data analysis process.

252 3.3 Driving simulator

253 A fixed-base open cab driving simulator was used to conduct the experiment. The driving 254 simulator had three LED screens displaying a 150° horizontal field of view. The driving 255 simulator was equipped with completely functioning controls (steering wheel, manual gear box, accelerator, brake, and clutch pedal) and produced simulated traffic and vehicle engine 256 257 sound. Two different software were available in the driving simulator namely SimVista and 258 SimCreator to develop static and dynamic events, respectively. The driving behavior data was 259 continuously recorded at 120 hz (Choudhary et al., 2020; Pawar and Velaga, 2021b; Yadav and 260 Velaga, 2021, 2020).

261 3.4 Participants

262 The sample contained 97 participants aged between 18 to 53 years and their descriptive 263 statistics are presented in Table 2 (Pawar et al., 2020; Pawar and Velaga, 2021a, 2020). The 264 mean age of the participants was 28.49 years (standard deviation: 7.92 years). The sample 265 consisted of 71.13 % male participants and 28.87 % female participants. The mean driving 266 experience and annual mileage of the participants were 7.38 years (standard deviation: 7.16 267 years) and 26.09 kms/1000 (standard deviation: 45.44 kms/1000), respectively. Among 97 268 participants, 29 were male professional car drivers working for a private transport company. 269 The participants were investigated about the overnight sleeping hours and exercise habits to 270 examine its influence on driver decisions. The mean overnight sleeping hours of the 271 participants was 6.36 with a standard deviation of 1.84. Around 41 % participants were 272 observed to be physically active (minimum 5 days of physical exercise for at least 30 mins 273 (Haskell et al., 2007)) whereas remaining 59 % reported being physically inactive. Table 2 274 presents descriptive statistics of the questionnaire data collected from the participants before 275 the start of the actual experiment.

Variable (Type)	Category	Mean (SD)	Percentage
Driver demographics			
Age in years (Con)	-	28.49 (7.92)	-
Gender (Cat)	Male	-	71.13
	Female	-	28.87
Physiological characteristics			
Overnight sleeping hours (Con)	-	6.36 (1.84)	-
Regular exercise (Cat)	Yes	-	41.23
	No	-	58.77
Driving history			
Professional driver (Cat)	Yes	-	29.90
	No	-	70.10
Annual mileage in kms/1000 (Con)	-	26.09 (45.44)	-
Simulator output data			
Approach speed in m/s at the onset of yellow	-	18.02 (32.54)	-
signal (Con)			
Mean acceleration in m/s ² (Con)	-	-1.88 (1.37)	-
Distance in meters of the driver from the stop	-	79.33 (52.21)	-
line of the intersection (Con)			
Reaction time in seconds (Con)	-	1.16 (0.83)	-

276 Table 2 Descriptive statistics of the data obtained through questionnaire and driving simulator

277 SD = Standard Deviation; kms = kilometers; Con = Continuous; Cat = Categorical.

278 3.5 Design of traffic signals

This study focused on analyzing drivers' acceleration behavior and crossing decisions at signalized intersections. A mixed 2*2*2 design was considered for the study. In total, four 281 signalized intersections were designed to study the effects of two distinct time to stop line 282 values (4 s and 6 s) (Choudhary and Velaga, 2019; Pathivada and Vedagiri, 2019) at two 283 different roadways (four-lane and two-lane undivided carriageway) for two types of maneuvers 284 (straight and right-turn). Time to stop line is the time required for a driver to stop the vehicle 285 based on their speed and distance from the stop line at the onset of yellow signal (Haque et al., 286 2016). Time to stop line is inclusive of yellow signal duration. Time to stop line is considered 287 in driving simulator studies instead of yellow signal duration because drivers are given an equal 288 chance of crossing as well as stopping at the intersection after the onset of yellow signal (Hussain et al., 2020a). The time to stop line values were selected based on literature as 289 290 presented in Table 1. From Table 1, it can be observed that yellow signal duration throughout 291 the previous studies was in the range of 2 seconds to 4 seconds (Ali et al., 2021; Dogan et al., 292 2011; Hussain et al., 2020a). Further, various field studies conducted over the years revealed 293 that yellow interval provided in the actual traffic conditions ranged in between 3 seconds to 6 294 seconds (Mishra and Zhu, 2015; Pathivada and Vedagiri, 2021). Moreover, Manual on Uniform 295 Traffic Control Devices (2009) suggests a yellow change interval in the range of 3 to 6 seconds 296 and Indo-HCM (2017) also advocates 3 seconds of yellow time. Due to all these reasons, 3 297 seconds of yellow time duration was incorporated in this study. Further, it was observed that 298 drivers mostly stopped at the intersection when they were 3 seconds away from the stop line 299 and crossed the intersection when they were 1 second away from the stop line (Pathivada and 300 Vedagiri, 2021; Rodegerdts et al., 2008). Thus, 3 seconds of yellow signal duration was 301 provided with an additional 1 second and 3 seconds making time to stop line values as 4 302 seconds and 6 seconds, respectively to study drivers' crossing decisions at signalized 303 intersections under time pressure driving conditions.

304 It should be noted that India follows left-side driving. Due to this reason, right-turn 305 maneuver becomes critical and thus, was considered in the study. The time to stop line was 306 determined using driver's distance from the stop line and instantaneous speed. A script was 307 programmed in such a way that the traffic signal turned from green to yellow when the 308 estimated time to stop line (ratio of distance from the stop line to the instantaneous speed) was 309 equal to 4 s or 6 s as per the assigned value to that specific intersection (Choudhary and Velaga, 310 2019). There was no ambient traffic in the vicinity of the signalized intersection. This was done 311 to avoid the interaction between driver and ambient traffic which might influence driver's 312 decision (Ali et al., 2021; Choudhary and Velaga, 2019; Haque et al., 2016). Further, the drivers 313 were not given any prior indication of signal change and were free to take the decision (stop or

- 314 cross the intersection after the onset of yellow signal) as per their perception and understanding.
- 315 Fig. 2 illustrates the schematic of signalized intersection as per time to stop line values
- 316 considered for the current study.



317

Fig. 2 Schematic of a signalized intersection representing onset of yellow signal when the
driver was 4 s and 6 s away from the stop line

320 3.6 Statistical approach

321 Three different statistical modeling techniques such as random forest, mixed effects 322 multinomial logit model, and generalized linear mixed model were considered for analyzing 323 drivers' acceleration behavior and crossing decision at signalized intersections. The 324 acceleration behavior was categorized into four different parts (detailed explanation in section 325 4.1) and random forest model was adopted to identify the explanatory variables governing 326 acceleration behavior (Jahangiri et al., 2016). Further, mixed effects multinomial logit model 327 was incorporated to establish a relationship between acceleration behavior and the significant 328 explanatory variables identified using random forest model (Wu et al., 2017). In the end,

drivers' crossing decisions were modeled using generalized linear mixed model for estimating crossing probability with respect to different time pressure driving conditions, acceleration behavior, and other explanatory variables (Choudhary and Velaga, 2020). A 10 % significance level was considered for examining the effects of explanatory variables on acceleration behavior and crossing decisions.

334 3.6.1 Random forest

335 A random forest approach, an ensemble learning technique, was used to identify the 336 factors based on variable importance ranking (Breiman, 2001). A random forest model is 337 developed based on two important features: total number of trees in the forest and number of 338 input variables at each split (Yu et al., 2019b). The data is randomly divided into two parts 339 namely training and testing datasets in 70:30 ratio for developing forest and estimating 340 prediction error (Harb et al., 2009). Bootstrap sampling technique is used for drawing samples 341 to develop decision trees from the training dataset (Sarkar et al., 2021). All the decision trees 342 are developed using random feature selection from the drawn samples, also known as bag 343 dataset (Yu et al., 2019a). Finally, the predictions are done using the developed random forest 344 on Out-Of-Bag (OOB) dataset (samples from training dataset which are not used during the development of decision trees) and testing dataset (Breiman, 2001; Ma et al., 2020). The 345 346 accuracy of the developed random forest model is determined based on the OOB error and 347 prediction error obtained from OOB dataset and testing dataset, respectively (Breiman, 2001; 348 Yang et al., 2019). Generally, a random forest is considered as an appropriate model if the 349 prediction accuracy of testing dataset is more than OOB dataset. In the end, random forest 350 model provides variable importance ranking in terms of permutation importance (Mean 351 Decrease Accuracy) and Gini importance (Mean Decrease Gini) (Jahangiri et al., 2016; Yu et 352 al., 2019a). The Mean Decrease Accuracy is estimated through OOB dataset and displays the 353 degradation of model without each variable. The Mean Decrease Gini is calculated using 354 training dataset and determines the purity of nodes at the end of decision tree without each 355 variable. A higher value for Mean Decrease Accuracy and Mean Decrease Gini indicates that 356 a variable has higher importance in portioning the data into classes (Wu et al., 2017).

The random forest classifier is basically used for dimensionality reduction or feature selection (Widmann and Silipo, 2015; Ye et al., 2018). The feature selection can be easily done through variable importance ranking. High score of mean decrease accuracy and mean decrease Gini indicates that the particular variable plays a substantial role in predicting the outcome 361 (Widmann and Silipo, 2015). Thus, a random forest classifier is used to predict the likelihood

362 of the dependent variable and parametric regression techniques are used to examine the impact

363 of the input variables on the dependent variable (Wu et al., 2017).

364 3.6.2 Mixed effects multinomial logit model

A mixed effects multinomial logit model was used to predict the probability of the acceleration behavior (four different categories) during a particular event. Repeated observations were collected from the same drivers in NTP, LTP, and HTP driving conditions. Therefore, mixed effects (fixed and random effects) were considered to incorporate between and within subject variations for predicting the probability of acceleration behavior. A mixed effects multinomial logit model can be expressed as follows (Hedeker, 2003; Wu et al., 2017):

371
$$p_{ij} = P\left(y_{ij} = \eta \middle| \rho\right) = \frac{\exp(\beta_{0\eta} + \beta_{\eta} X_{ij\eta} + V_{ij\eta} + \varepsilon_{ij\eta})}{1 + \sum_{\chi} \exp(\beta_{0\chi} + \beta_{\eta} X_{ij\chi} + V_{ij\chi} + \varepsilon_{ij\chi})} (\forall \eta \neq \chi)$$
(1)

372 where, y is the dependent variable, P is the probability of i^{th} participant (1 to 97) in j^{th} 373 time pressure driving condition, η is the category of acceleration behavior, ρ is the random 374 effects, β is the parameter estimate, V_{ij} is the vector of random disturbances and ε_{ij} is the random 375 error independently and identically distributed (Wu et al., 2017). The parameter estimates for 376 mixed effects multinomial logit model were determined using iterative maximum likelihood 377 solution.

378 3.6.3 Generalized linear mixed model

A generalized linear mixed model was considered for estimating the probability of crossing decision according to acceleration behavior and time to stop line under time pressure driving conditions. The mixed effects were considered for obtaining unbiased estimates in the model due to repeated data collection (Gupta et al., 2021; Mannering et al., 2016). A generalized linear mixed model can be formulated as follows (Yadav and Velaga, 2019):

384
$$g(y_{ij}) = \beta_0 + \beta * X_{ij} + V_{ij} + \varsigma_{ij}$$
 (3)

385 where, g is the link function ς is the unobserved random error with mean zero and 386 constant variance (Xiong et al., 2007). A logit link function is generally used to establish a relationship between dependent variable and explanatory variables for a binary response as shown below (Choudhary and Velaga, 2018):

$$390 \quad \log\left[\frac{P(y_{ij}=1)}{1-P(y_{ij}=1)}\right] = \beta_0 + \beta * X_{ij} + V_{ij} + \varsigma_{ij}$$
(4)

391 where, log is the natural logarithm. Thus, probability can be estimated as follows: 392 $P(y_{ij} = 1) = \frac{1}{1 + exp - (\beta_0 + \beta * X_{ij} + V_{ij})}$ (5)

The crossing decision was a binary variable where y = 1 was considered for crossing decision and y = 0 was considered for stopping decision. The parameter estimates for GLM model were determined using maximum simulated likelihood approach (Drukker, 2006).

396 4. Data analysis and results

397 4.1 Acceleration behavior of the drivers after the onset of yellow signal under time pressure

398 The current study attempts to explore the impact of different time pressure driving 399 conditions on acceleration behavior of the drivers while driving through a signalized 400 intersection when the signal turns from green to yellow. Thus, the acceleration behavior was 401 observed from the point when the drivers reacted to the change in traffic signal from green to 402 yellow. The acceleration behavior was categorized into four different parts as nominal variable: 403 (a) continuous deceleration (drivers continuously decelerating till they stop the vehicle); (b) 404 acceleration-deceleration (drivers initially accelerating but finally decelerating to stop the 405 vehicle); (c) deceleration-acceleration (drivers initially decelerating but finally accelerating to 406 cross the intersection); and (d) continuous acceleration (drivers continuously accelerating to 407 cross the intersection). The driver's accelerator pedal and brake pedal applications were 408 assessed simultaneously for extracting acceleration behavior data. Application of accelerator 409 pedal was considered as acceleration and release of accelerator pedal with application of brake 410 pedal was considered as deceleration. In total, 1,164 encounters were observed at four signalized intersections for three different time pressure driving conditions from 97 411 412 participants. There were 874 (75 %), 109 (9 %), 113 (10%), and 68 (6%) continuous 413 deceleration, acceleration-deceleration, deceleration-acceleration, and continuous acceleration 414 encounters, respectively during NTP, LTP, and HTP driving conditions as shown in Fig. 3.



416 Fig. 3 Percentage of different acceleration encounters observed throughout the 417 experiment.

418 4.2 Explanatory variables

415

The explanatory variables or input variables can be divided into five broad categories: driver demographics and driving history, physiological characteristics, driving condition, scenario configuration, and driving attributes. The details of the variables as per each category are presented below:

423 Driver demographics and driving history: Driver's age, gender, driving profession, driving
424 experience, and annual mileage were considered under this category.

425 Physiological characteristics: Driver's exercise habits and overnight sleeping hours were426 considered under this category.

427 Driving condition: This particular factor accounted the effect of time pressure imposed on the428 drivers in the form of NTP, LTP, and HTP.

429 Scenario configuration: The different configurations of the signalized intersections such as time

430 to stop line (4 s vs 6 s), type of maneuver (right turn vs straight), and number of lanes (two-

431 lane vs four lane) were considered in this category.

432 Driving attributes: Approach speed, reaction time, mean acceleration (measured from the point
433 when the drivers reacted to change in signal from green to yellow), and distance to stop line of
434 the intersection were considered under this category.

435 4.3 Identification of the factors governing acceleration behavior using random forest 436 approach

437 Overall, the current study had 1,164 valid samples from which 821 samples were 438 randomly selected for training dataset and remaining 343 samples were selected for testing 439 dataset. Initially, all 15 input variables (described in section 4.2) were considered while 440 developing random forest model. This was done because the random forest model can mitigate 441 the multicollinearity issue and effectively use the variables using random feature selection (square root of total number of variables at each split) and numerous decision trees (Yu et al., 442 443 2019b). Thus, at the initial stage, a random forest model was developed, which comprised of 444 1,000 decision trees and 3 input variables at each split and the prediction results are presented 445 in Table 3. The OOB error rate was 18.25 % providing an accuracy of 81.75 % while predicting 446 acceleration behavior of the drivers. The testing dataset provided an accuracy of 80.41 %. The 447 overall analysis indicated that the developed random forest model provides decent accuracy 448 while predicting the acceleration behavior, however, the model can be further improvised by 449 tuning number of decision trees and input variables. Thus, the class error and OOB error was 450 estimated for different combinations of decision trees and input variables. It was observed that 451 class error was constant after 900 decision trees and the OOB error was lowest for 6 input 452 variables. Therefore, a new random forest model was developed with 900 decision trees and 6 453 input variables at each split and the results are presented in Table 4

454 The OOB error of the improvised random forest model was 17.64 %, i.e., the accuracy 455 was 82.36 %. Further, the prediction accuracy through testing dataset increased from 80.41 % 456 to 83.09 %. Thus, it can be concluded that the improvised random forest model with 900 457 decision trees and 6 input variables performs well while predicting acceleration behavior of the 458 drivers. Finally, the variable importance ranking was estimated with two measures, Mean 459 Decrease Accuracy and Mean Decrease Gini as shown in Fig. 4. The variable importance 460 ranking signified the accuracy of the model performance associated with each variable. 461 Generally, the variables with Mean Decrease Accuracy and Mean Decrease Gini values less than 10 are considered having very low influence and can be eliminated from the model (Yu et 462 463 al., 2019b). It can be observed that all the variables had Mean Decrease Accuracy (Fig. 4a) and

464 Mean Decrease Gini (Fig. 4b) values more than 10. This represented the significance of the input variables while predicting acceleration behavior. The mean acceleration and approach 465 466 speed ranked top two in both the measures indicating their importance while predicting 467 acceleration behavior. Apart from these two variables, time pressure driving conditions, 468 reaction time, driver's distance from the intersection when the signal turned from green to 469 yellow, time to stop line, driving experience, and age significantly influenced the prediction of 470 acceleration behavior. Thus, all these variables can be considered for modeling acceleration 471 behavior using mixed effects multinomial logit model.

472 Table 3 Confusion matrix for prediction of acceleration behavior using random forest model

Training set							
Predicted	CD	AD	DA	СА	Class error	OOB Error rate	Accuracy
CD	614	5	2	2	0.014	18.25 %	81.75 %
AD	41	13	7	12	0.82		
DA	44	4	17	13	0.78		
CA	4	10	6	28	0.41		
Testing set							
Predicted	CD	AD	DA	CA	Class error	Accuracy	Confidence interval
CD	251	21	26	0	0.15	80.41 %	75.80 %,
AD	0	6	3	1	0.40		84.48 %
DA	0	3	4	5	0.66		
CA	0	6	2	14	0.36		

473 CD = Continuous Deceleration; AD = Acceleration-Deceleration; DA = Deceleration-Acceleration; CA =
474 Continuous Acceleration; OOB = Out-Of-Bag.

475

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Table 4 Confusion matrix for prediction of acceleration behavior using random forest model
with 900 decision trees and 6 input variables

Training set							
Dradiatad	CD	AD	DA	CA	Class	OOB Error	Agauraay
Treulcieu	CD	AD	DA	CA	error	rate	Accuracy
CD	611	5	5	2	0.019	17.64%	82.36%
AD	37	11	12	13	0.84		
DA	34	6	24	14	0.69		
CA	1	9	7	31	0.35		
Testing set							
Prodicted	CD	AD	ПА	C۸	Class	Accuracy	Confidence
Truccu	CD	AD	DA	CA	error	Accuracy	interval
CD	251	18	21	0	0.13	83.09%	78.64%,
۸D	0	10	1	1	0.16		86.86%
AD	0	10	1	1	0.10		
DA	1	2	11	6	0.45		
CA	0	6	2	13	0.38		

 $480 \qquad CD = Continuous Deceleration; AD = Acceleration-Deceleration; DA = Deceleration-Acceleration; CA =$

481 *Continuous Acceleration; OOB = Out-Of-Bag.*





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484



486 Fig. 4 Variable importance ranking in terms of (a) Mean Decrease Accuracy and (b) Mean 487 Decrease Gini

488 4.4 Modeling acceleration behavior using mixed effects multinomial logit model

489 A mixed effects multinomial logit model was considered to analyze four distinct variations observed in acceleration behavior (Li et al., 2020; Pani et al., 2020; Wu et al., 2017). 490 491 Continuous deceleration was considered as the reference category for modeling acceleration behavior using mixed effects multinomial logit model. The explanatory variables ranking in 492 493 descending order of importance, obtained through random forest's variable importance 494 analysis were involved in acceleration behavior prediction model. Table 5 presents the 495 estimation results of the mixed effects multinomial logit model with goodness of fit. Time 496 pressure driving conditions (NTP as reference category), Time to stop line (6 s as reference 497 category), and mean acceleration were observed to have significant impact on different 498 acceleration behavior of the drivers. It is interesting to observe that the coefficients of the model 499 for deceleration-acceleration, acceleration-deceleration, and continuous acceleration had same effect with different magnitude. The intercept of the model results had negative effect on 500 501 acceleration behavior. This signified that the possibility of encountering deceleration-502 acceleration, acceleration-deceleration, and continuous acceleration as compared to continuous deceleration when the signal changed from green to yellow were 43 %, 44 %, and 40.3%, 503 504 respectively.

505 Further, the model results revealed that drivers adopting acceleration-deceleration 506 behavior instead of continuous deceleration behavior was higher under LTP (70%) than HTP 507 (63%) driving conditions. The likelihood of drivers undergoing deceleration-acceleration and 508 continuous acceleration rather than continuous deceleration patterns were higher during HTP 509 than LTP driving conditions. In general, there were high chances of encountering deceleration-510 acceleration (123%) as compared to continuous acceleration (77%) and acceleration-511 deceleration (63.5%) in lieu of continuous deceleration behaviors under HTP driving condition 512 when the signal turned from green to yellow. The mean acceleration showed substantial positive effect while predicting acceleration behavior. It can be observed that 1 m/s² increment 513 in mean acceleration increased the odds of encountering acceleration-deceleration, 514 515 deceleration-acceleration, and continuous acceleration rather than continuous deceleration 516 behaviors by 46%, 47%, and 60%, respectively. Moreover, the odds of driver adopting deceleration-acceleration, continuous acceleration, and acceleration-deceleration over 517 continuous deceleration were lower by 35%, 33%, and 26%, respectively when TSL was 6 s 518 than 4 s. Other explanatory variables (irrespective of variable importance ranking) showed no 519 520 significant effect on acceleration behavior under different time pressure driving conditions.

	Acceleration-deceleration					Deceleration-acceleration				Continuous acceleration		
Parameters	β	Exp(β)	SE	t-stat	β	Εχρ(β)	SE	t-stat	β	Exp(β)	SE	t-stat
Intercept	-0.55	0.58	0.19	-2.89***	-0.58	0.56	0.19	-3.02***	-0.51	0.60	0.19	-2.64***
LTP	0.53	1.70	0.20	2.54**	0.54	1.72	0.21	2.50**	0.48	1.62	0.22	2.20**
HTP	0.49	1.63	0.20	2.31**	0.80	2.23	0.21	3.82***	0.57	1.77	0.22	2.59**
Mean acceleration	0.38	1.46	0.16	5.92***	0.39	1.47	0.06	6.11***	0.47	1.60	0.06	6.92***
Time to stop line: 6 s	-0.30	0.74	0.064	-1.793*	-0.43	0.65	0.16	-2.59**	-0.40	0.67	0.17	-2.28**
Goodness-of-fit												
-2 log pseu likelihoo	ldo d	AICC		BIC								
9,506.77	,	9,512.79	9,	527.91								

Table 5 A mixed effects multinomial logit model results for predicting acceleration behavior (reference: continuous deceleration)

 $\beta = Estimate; Exp = Exponential; SE = Standard Error; LTP = Low Time Pressure; HTP = High Time Pressure; AICC = Akaike Information Corrected Criterion; BIC = 523 Bayesian Information Criterion; *p < 0.10; **p < 0.05; ***p < 0.01.$

524 Fig. 5 was plotted to acquire better insights on acceleration-deceleration and deceleration-525 acceleration behaviors with respect to continuous deceleration behavior. According to 526 AASHTO (2011), most of the vehicle-braking systems are capable of providing a comfortable deceleration rate of 3.4 m/s². Numerous studies documented in the literature suggested a 527 threshold of 3.4 m/s² for comfortable deceleration to stop the vehicle (Guido et al., 2012; Kuang 528 529 and Qu, 2014; Mahmud et al., 2017; Tomar et al., 2020). Therefore, acceleration-deceleration 530 and deceleration-acceleration behaviors were compared at -3.4 m/s^2 mean acceleration. It can be observed that drivers under NTP and LTP driving conditions with mean acceleration of -3.4 531 m/s² when time to stop line was 6 s were more likely to exhibit acceleration-deceleration (10.4 532 %) than deceleration-acceleration (8.8 %) behaviors instead of continuous deceleration 533 behavior as shown in Fig. 5a and Fig. 5b, respectively. No significant difference can be 534 535 observed in between acceleration-deceleration (13.6 %) and deceleration-acceleration (13.1 %) 536 probabilities under NTP and LTP driving conditions when the drivers were 4 s away from the 537 intersection (Fig. 5a and Fig. 5b). Further, acceleration behavior of the drivers varied 538 significantly under HTP driving condition. The drivers under HTP were observed to undergo 539 deceleration-acceleration behavior as compared to acceleration-deceleration and continuous 540 deceleration behaviors as shown in Fig. 5c. The drivers under HTP had high likelihood of 541 adopting deceleration-acceleration behavior when time to stop line was 4 s than 6 s. From Fig. 542 5c, it can be observed that the odds of drivers adopting deceleration-acceleration behavior (25.1 % and 17.8 % when time to stop line were 4 s and 6 s, respectively) over acceleration-543 deceleration behavior (20.5 % and 16.1 % when time to stop line were 4 s and 6 s, respectively) 544 545 were substantially higher under HTP driving condition. Thus, it can be clearly understood that driving strategy of the drivers altered as per time pressure driving conditions and time to stop 546 547 line at signalized intersections.





Fig. 5 Graphical representation of drivers' acceleration-deceleration and deceleration-acceleration
 behaviors in terms of continuous deceleration behavior under (a) NTP, (b) LTP, and (c) HTP
 driving conditions

558 4.5 Modeling crossing decisions

559 Throughout the driving session, there were 875 stop decisions and 289 cross decisions at 560 signalized intersections as mentioned in section 4.1. All the drivers adopting continuous 561 acceleration were found to cross the intersection (23.18 % of total cross decisions) whereas 39.10 %, 32.52 %, and 5.2 % of the total crossing decisions were observed during deceleration-562 563 acceleration, acceleration-deceleration, and continuous deceleration behaviors. On the other 564 hand, 98.17 % of the total stop decisions were due to drivers' continuous deceleration and the 565 remaining 1.83 % stop decisions comprised of acceleration-deceleration (1.71 %) and 566 deceleration-acceleration (0.12 %) encounters. Here, it should be noted that around 2 % drivers 567 undergoing continuous deceleration failed to stop before the stop line of the intersection. The 568 possible reason might be the inadequate deceleration after the onset of yellow signal. The 569 drivers failing to stop before the stop line of the intersection might have decided to cross the intersection due to the intuition of unsafe driving environment while stopping the vehicle aftercrossing stop line of the intersection (Choudhary and Velaga, 2020).

Drivers' crossing decisions were analyzed using Generalized Linear Mixed (GLM) 572 573 model with logit link function and the results are presented in Table 6. The GLM model results 574 revealed that the likelihood of crossing a signalized intersection increased by 2.73 times and 575 4.26 times when the drivers were under LTP and HTP driving conditions, respectively as 576 compared to NTP driving condition. Further, the probability of stopping reduced by 7.02 and 577 10.20 when the drivers adopted acceleration-deceleration and deceleration-acceleration 578 patterns in lieu of continuous deceleration. The crossing probability was also influenced by time to stop line and age. The odds of crossing the signalized intersection reduced by 62%579 580 when the signal turned from green to yellow 6 seconds prior to the stop line of the intersection. 581 Moreover, the crossing probability reduced by 74 % with every 1-year increment in age.

582 Fig. 6 represents crossing probability of the drivers according to acceleration behavior, 583 time pressure driving conditions, and drivers' age. It can be observed that crossing probability 584 decreased continuously with increment in age. Thus, it can be anticipated that the crossing 585 probability will continuously decrease beyond the age limit considered in this study. The 586 crossing probability of the drivers adopting deceleration-acceleration behavior was more when time to stop line was 4 s than 6 s as shown in Fig. 6. Further, a substantial decrement in crossing 587 588 probabilities can be observed with increment in age when the drivers adopted acceleration-589 deceleration behavior instead of deceleration-acceleration behavior. This implied that the effect 590 of signal change from green to yellow was relatively more acute with increment in age when 591 time to stop line was 6 s than 4 s steering drivers to adopt acceleration-deceleration behavior 592 instead of deceleration-acceleration behavior.

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Parameters	<mark>Estimate (β)</mark>	<mark>Exp(β)</mark>	<mark>SE</mark>	<mark>z-value</mark>						
Intercept	<mark>-4.34</mark>	<mark>0.01</mark>	<mark>0.70</mark>	<mark>-6.16***</mark>						
LTP	<mark>1.01</mark>	2.73	<mark>0.56</mark>	<mark>1.79*</mark>						
HTP	<mark>1.45</mark>	<mark>4.26</mark>	<mark>0.55</mark>	<mark>2.62***</mark>						
Acceleration-deceleration	<mark>7.02</mark>	7.02 1119.90		<mark>9.69***</mark>						
Deceleration-acceleration	<u>10.20</u>	10.20 27038.04		<mark>7.91***</mark>						
Time to stop line: 6 s	<mark>-0.94</mark>	-0.94 0.38		<mark>-2.18**</mark>						
Age	<mark>-1.31</mark>	<mark>-1.31</mark> 0.26		<mark>-2.27**</mark>						
Goodness-of-fit of the model										
df Log-l	ikelihood	AIC		BIC						
8 -1	10.78	237.57	2	<mark>77.57</mark>						

Table 6 Estimates of generalized linear mixed model for crossing probability at signalized intersections

600 Exp=Exponential; SE=Standard Error; df=degrees of freedom; AIC=Akaike Information Criterion; 601 BIC=Bayesian Information Criterion; *p < 0.10; **p < 0.05; ***p < 0.01.



602



Fig. 6 Crossing probability of the drivers according to age while adopting acceleration deceleration and deceleration-acceleration behaviors when time to stop line was (a) 4 s and
 (b) 6 s under time pressure driving conditions

609 5. Discussion

610 The current study investigated acceleration behavior and crossing decision of the drivers 611 at the onset of yellow signal under increasing time pressure driving conditions. The acceleration behavior of the drivers was characterized into four distinct categorizes. Random forest model 612 613 was used to identify the factors influencing acceleration behavior of the drivers. The identified factors were used as explanatory variables in mixed effects multinomial logit model for 614 615 predicting acceleration behavior. In the end, a generalized linear mixed model was developed 616 for estimating crossing probability of the drivers according to time pressure driving conditions, 617 acceleration behavior, and gender.

The random forest model revealed that driving attributes such as approach speed, reaction time, and distance to stop line of the intersection showed significant effect on acceleration behavior of the drivers. Previous research revealed that drivers who were near to the intersection stop line with high approach speed, reacted swiftly to cross the intersection when the signal turned from green to yellow. Here, it should be noted that high driving speed will 623 result in lower acceleration capability of the vehicle which might influence driver's decision 624 forcing him/her to alter acceleration behavior (Palat and Delhomme, 2016). The research 625 conducted by Lee et al. (2002) showed that swift reaction to the event provides driver with 626 additional time to evaluate his/her decision and accordingly alter driving behavior (Lee et al., 627 2002). Nevertheless, numerous studies showed that drivers near to the intersection, driving at 628 high speed were more likely to cross the intersection (Choudhary and Velaga, 2019; Haque et 629 al., 2016; Pathivada and Vedagiri, 2019). This can be attributed to the fact that drivers may end 630 up in the dilemma zone, where they cannot safely stop at the intersection due to the high driving 631 speed and inadequate space availability for halting the vehicle before the stop line of the 632 intersection (Ali et al., 2021; Elmitiny et al., 2010).

633 The random forest model showed that mean acceleration was the most important input factor for predicting acceleration behavior. The acceleration behavior was characterized into 634 635 four distinct categorizes by simultaneously observing accelerator pedal and brake pedal application after the driver's reaction to the onset of yellow signal. The mean acceleration 636 637 values were also extracted from the point when the drivers reacted to change in signal from 638 green to yellow. Thus, mean acceleration values were representative of acceleration behavior. 639 A random forest model was developed without mean acceleration and very low prediction 640 accuracy was observed. Further, in reality, it is not possible to directly obtain the change in 641 acceleration behavior (for example acceleration-deceleration or deceleration-acceleration 642 behaviors). Nevertheless, mean acceleration of the drivers can be easily recorded. Due to these 643 reasons, mean acceleration was considered while modeling acceleration behavior of the drivers.

644 The mixed effects multinomial logit model result revealed that drivers with mean deceleration rate of 3.4 m/s² had around 89 – 91 %, 83 – 85 %, 82 – 84 % likelihood of adopting 645 continuous deceleration behavior under NTP, LTP, and HTP driving conditions, respectively 646 647 when they were 6 s away from the intersection. Similarly, the possibility of adopting continuous deceleration behavior with mean deceleration of 3.4 m/s² under NTP, LTP, and HTP driving 648 conditions when the drivers were 4 s away from the intersection were 86 - 87 %, 79 - 80 %, 649 75 - 79 %, respectively. Thus, it can be concluded that drivers had 75 - 91 % possibility of 650 adopting continuous deceleration when the mean deceleration was 3.4 m/s² to comfortably stop 651 652 the vehicle. Further, previous literature on signalized intersection revealed that deceleration 653 rate and crossing probability decreased with increment in time to stop line values. The stopping 654 probability (100 - crossing probability) of the drivers adopting continuous deceleration were

98.72 %, 96.59 %, and 94.77 % under NTP, LTP, and HTP driving conditions, respectively when they were 4 s away from the intersection. The stopping probability increased up to 99.50 %, 98.64 %, and 97.90 % under NTP, LTP, and HTP driving conditions, respectively when the drivers were 6 s away from the intersection. Thus, it can be concluded that drivers who were far from the intersection were more likely to adopt continuous deceleration to stop at the intersection.

In the current study, female drivers showed high likelihood of crossing the intersection as compared to male drivers. The study conducted by Ali et al. (2021) and Haque et al. (2016) revealed that female drivers were less likely to stop at the onset of yellow signal. The current study showed that male drivers were quick to react to the signal change (green to yellow) as compared to female drivers (t (525.45) = 3.69; p-value < 0.01). Thus, slow response of female drivers to signal change might be the reason behind high crossing probability than male drivers (Haque et al., 2016).

668 **6.** Conclusions

This study examined three research hypotheses as stated in section 2.4. The mixed effects 669 670 multinomial logit model and generalized linear mixed model results indicated that time pressure 671 had a significant effect on acceleration behavior and crossing decision of the drivers at the onset 672 of yellow signal. The driver decisions were also observed to vary significantly as per time to 673 stop line values and driver demographics. Based on the results obtained through this study, it 674 can be concluded that time pressure alters acceleration behavior of the drivers leading to risky driving decisions which can have serious safety critical situations as compared to normal 675 676 driving. Therefore, drivers should be made aware of severe consequences of involving in 677 deliberate or accidental red-light violations resulting from crossing decisions taken during the 678 onset of yellow signal. Further, it is advisable that drivers should gradually stop at signalized 679 intersection after the onset of yellow signal, irrespective of driving condition, to avoid 680 hazardous traffic situations (Mathew, 2009).

681 **7. Research contribution and implication**

682 The current study contributed to the existing literature in four main facets:

- 683 (i) As per the authors' best knowledge, this is the first study that evaluated the influence of
- 684 increasing time pressure driving conditions on acceleration behavior and crossing

decision at signalized intersections. Most of the existing studies directly evaluated
drivers' crossing decision without considering possible implications of acceleration
behavior. Thus, the current research work makes a significant contribution to the existing
literature by modeling acceleration behavior which can be effectively used for predicting
crossing probability of the drivers.

- 690 (ii) This study provides insights on crossing probabilities in normal and time pressure driving 691 conditions. The results obtained through this study can be used to augment the design of 692 traffic signal timings (yellow and all red timings). This can be achieved by identifying 693 the neighborhood with various types of built-up environments (commercial, corporate, 694 residential, recreational, etc.), socio-demographics, and transportation infrastructure. The 695 yellow and all red signal timings of the particular area having high chances of drivers 696 driving under time pressure can be investigated based on research methodology followed 697 in the current study and modified by performing similar analysis.
- 698 (iii) It is extremely difficult to detect acceleration behavior of the drivers in real-time or 699 naturalistic studies. The developed mixed effects multinomial logit model showed that 700 acceleration behavior can be successfully predicted using mean acceleration and time to 701 stop line values after the onset of yellow signal. Various studies showed that drivers 702 experience high dilemma when they were around 6 seconds away from the stop line of 703 the intersection (Pathivada and Vedagiri, 2021; Rakha et al., 2008). Thus, driving 704 strategies can be developed in terms of acceleration behavior for minimizing abrupt 705 acceleration-deceleration rates to reduce waiting time at intersections using Model 706 Predictive Control (MPC) and Vehicle- to-Infrastructure (V2I) communication systems 707 (He et al., 2021; Ubiergo and Jin, 2016). The MPC system can estimate the required 708 acceleration by collecting information of traffic signal timing from V2I system and 709 develop driving strategy for stopping or crossing the intersection when the drivers are 710 around 6 seconds away from the stop line of the intersection (Butakov and Ioannou, 2016; 711 He et al., 2021).
- (iv) Most of the signalized intersections in India are without countdown timer because of
 which drivers are subjected to sudden change in traffic signal. Normal reaction time
 considered in transportation research is 2.5 seconds whereas yellow signal provides 3
 seconds to take a decision (AASHTO, 2011; Indian Road Congress, 1976; Pathivada and
 Vedagiri, 2019). Thus, a particular driver has only 0.5 seconds for implementing his/her
 decision. From the current study, it can be understood that drivers react within 1.5
 seconds irrespective of driving condition and driver demographics. The swift reaction

719 720 from the drivers might be due to the fact that they are habitual to the sudden signal change and have developed themselves to take quick decisions (Pawar and Velaga, 2020).

721 8. Study limitations and future scope

722 The current study has some limitations which restrict generalization of the results. 3 723 s yellow signal duration was fixed for the current study. In future, researchers can vary the 724 yellow signal duration to check its influence on acceleration behavior and crossing decision 725 under time pressure driving conditions. The study invited all the eligible participants; 726 however, the sample was dominated by male drivers as compared to female drivers. This 727 might be due to the fact that India has very low female driving population (Ministry of 728 Road Transport & Highways, 2019). Further, drivers above the age of 60 years experienced 729 simulator sickness and therefore were excluded from the study. The data collection was 730 conducted in the fixed order of NTP, LTP, and HTP driving conditions. The fixed order of 731 data collection is considered as a standard method in time pressure experimental studies 732 (Bertola et al., 2012; Gelau et al., 2011; Paschalidis et al., 2018; Pawar and Velaga, 2020; 733 Rendon-Velez et al., 2016). However, there exists a high chance of learning effects. 734 Therefore, a repeated measures ANOVA and post hoc test was conducted on reaction time 735 to examine the influence of learning effects. The results showed insignificant effects of 736 fixed order on reaction time (F (2, 774) = 2.237, p-value = 0.11). Thus, it can be concluded 737 that repeated data collection in fixed order had insignificant effects on driving behavior of 738 the drivers (Pawar and Velaga, 2020). The current study specifically focused on driving 739 behavior at the onset of yellow signal at four-legged signalized intersections. In future, 740 similar research can be carried on three-legged signalized intersections where driving 741 behavior during yellow signal and red signal can be examined under time pressure driving 742 conditions. A fixed-base driving simulator was used in this study to conduct the 743 experiments. In future, researchers can conduct experiments on moving-base driving simulators to obtain better insights on acceleration behavior of the drivers under various 744 745 time pressure driving conditions.

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