Highway Capacity Software 2010

Version 6.50 April 8, 2014



Highway Capacity Software

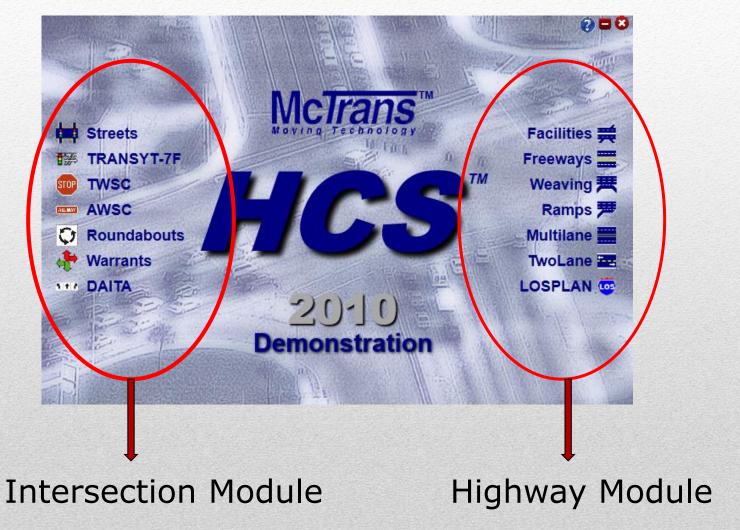
Developed and maintained by McTrans

- Originally founded by the FHWA in 1986 as the Center for Microcomputers in Transportation (McTrans)
- Now a full-service software support center, associated with the University of Florida

HCS 2010

- HCS 2010 implements the procedures defined in the Highway Capacity Manual (HCM) 2010 published by the Transportation Research Board (TRB)
- This release includes the new Streets module that combines the Signalized Intersections with the Urban Streets Segments, Facilities and Multimodal procedures.

Fourteen Modules



Street Module

Signalized Intersections

- Signal analysis
- Interchange analysis
- Multimodal analysis
- LOS

Capacity

- Defined for each lane group
 - Lane group: one or more lanes that accommodate traffic and have a common stop-line and traffic move together
- Lane group capacity: maximum rate of flow for the subject lane group that may pass through the intersection under prevailing traffic, roadway and signalized conditions

Traffic Conditions

- Approach volumes (left, through, right)
- Vehicle type
- Location of bus stops
- Pedestrian crossing flows
- Parking movement

- Roadway Conditions
 - Number and width of lanes
 - Grades
 - Lane use
 - Including parking lanes
- Traffic Signal Characteristics
 - Signal phasing
 - Signal timing
 - Type of control
 - Signal progression

- Delay experienced by a motorist includes many factors:
 - Signal control
 - Geometrics
 - Incidents

Total delay:

- Difference between actual travel time and ideal travel time
 - In the absence of traffic control, delay due to roadway geometries, incidents and when there are no vehicles on the road
- In HCS control delay is quantified
 - initial deceleration delay
 - Queue move-up time
 - Stopped delay
 - Final acceleration delay

Level of Service (LOS)

Defined in terms of delay as a function of

- driver discomfort
- Driver frustration
- Fuel consumption
- Lost travel time

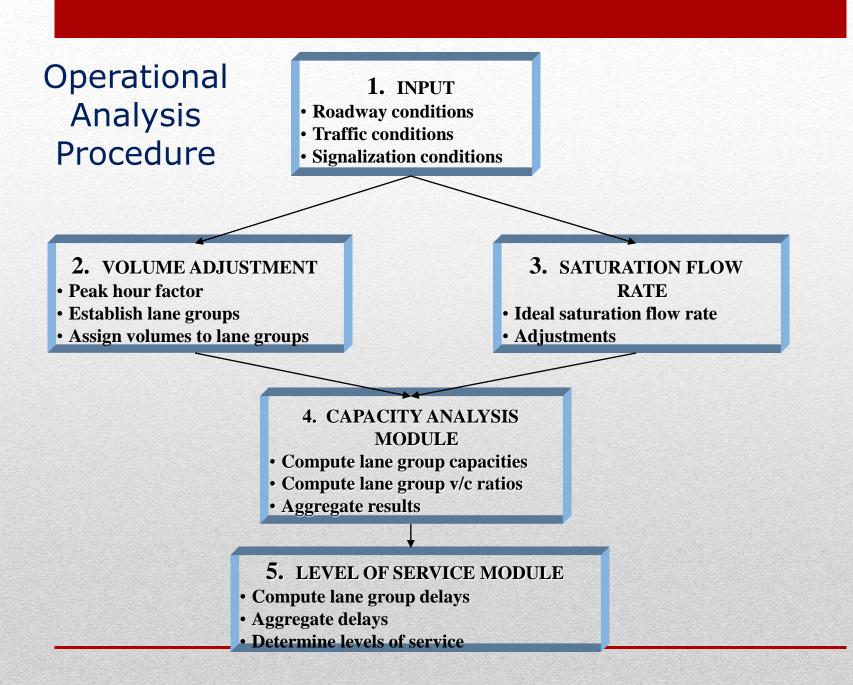
Level of Service (LOS)

- LOS criteria are stated in terms of average control delay per vehicle
 - Delay on signal control depends on
 - Quality of progression
 - Cycle length
 - Green ratio
 - V/c ratio for lane group
- Designated by letters A F

Level of Service (LOS)

LEVEL OF SERVICE	CONTROL DELAY PER VEHICLE (SEC/VEHICLES)
А	≤ 10
В	$>10 \text{ and } \le 20$
С	$>20 \text{ and } \le 35$
D	>35 and ≤ 55
E	>55 and ≤ 80
F	> 80

Source: Highway Capacity Manual 2010



Getting Started:

1. Open HCS 2010

2. Select the Streets (handles signals and signalized corridors)



Getting Started:

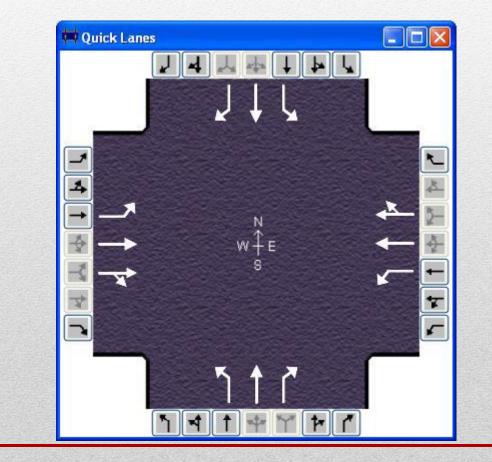
3. Complete The Quick Start Screen

luick Start			
Default Selections			
Number of Intersections	1 🗘	Cycle Length, s	100
Forward Direction	NB 🔽	Minimum Green, s	5
Number of Periods	1 🗘	Yellow Change, s	4.0
Analysis Duration, h	0.25	Red Clearance, s	1.0
Base Saturation Flow, pophpl	1900	Passage Time, s	2.0
Speed Limit, mi/h	45	Detector Length, ft	40
Template Help 🕜			Select Template
	<u>(</u>	<u>]K C</u> ancel	

4. Enter Information in the General Section

Ħ HCS 2010 Stree	ts - [Str	eet	s2.xus *]							
🖳 File View Ed	dit Win	idov	vs Reports	Help						
i 🗋 📂 🔚 🛃 🚺	🚦 Тлन 🔇)	k 🛍 🖊	Ø 🛑	N					
Classic Mode Visual	Mode									
General	DATA				_					
Urban Street	SR 42									
Intersection	SR 42 @ Forest Pkwy									
Description	Existing Geometry Design Year Traffic									
Data File	Streets2.	xus								
Forward Direction	NB	•	Area Type	Other	•					
Segment Length, ft			Duration	0.25						
All Segment L	engths		PHF 0.92							

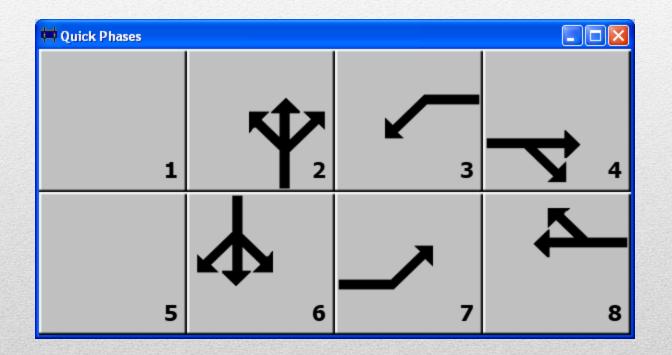
5. Enter Lane Configuration



6. Enter Traffic Data

Traffic												
	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Demand, veh/h	350	1315	285	385	740	125	130	320	240	225	780	310
Lane Width, ft	12.0	12.0	12.0	12.0	12.0	12.0	12.0	12.0	12.0	12.0	12.0	12.0
Storage Length, ft	350	0	0	300	0	0	190	0	300	200	0	370
Saturation, pc/h/ln	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Heavy Vehicles, %	10	10	0	10	10	0	10	10	10	10	10	10
Grade, %		-2			1			6			-2	
Buses, per h			0			0			0			0
Parking, per h	0	Ν -	0	0	Ν.*	0	0	Ν.*	0	0	Ν.*	0
Bicycles, per h		0			0			0			0	
Pedestrians, per h		0			0			0			0	
Arrival Type	3	3	3	3	3	3	3	3	3	3	3	3
Upstream Filtering (I)		8	1.00		VB 🔡	1.00	- I-N	IB 🔰	1.00	1.5	8	1.00
Initial Queue, veh	0	0	0	0	0	0	0	0	0	0	0	0
Speed Limit, mi/h		45			45			45			45	
Detector, ft	40	40	40	40	40	40	40	40	40	40	40	40
RTOR, veh/h			0			0			0			0

7. Enter Phasing data



8. Enter Timing

~ Timing	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Phase Split, s	15.0	40.0	15.0	40.0	0.0	45.0	0.0	45.0
Yellow Change, s	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Red Clearance, s	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Minimum Green, s	5	5	5	5	5	5	5	5
Lag Phase	📃 EL	ET	WL 📃	WT	NL	NT	SL	ST
Passage Time, s	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Recall Mode	Off 🝷	Off 💌	Off 💌	Off 💌	Off 💌	Mir 💌	Off 🝷	Mir 💌
Dual Entry	📃 EL	🖌 ET	WL 📃	🔽 WT	NL	V NT	SL	🖌 ST
Dallas Phasing	E/W	N/S	S	imultane	ous Gap	🔽 E/W	🔽 N/S	

	245 517	~ ~	 ↓↓			
Green	40.0	10.0	35.0	0.0	0.0	0.0
Yellow	4.0	4.0	4.0	0.0	0.0	0.0
Red	1.0	1.0	1.0	0.0	0.0	0.0

9. Enter the **Detailed** Data

General				Intersection -															_
Analyst								EBL	EBT				WBR					SBT	
Agency/Co				Lanes				1	2	1	1	2	1	1	2	0	1	2	0
Date V	/ed, September 07,	2011		Shared Lane													V		
Time Period				Percent Turns	in Shared La	ane		0		0	0		0	0	Į	0	0	Į	0
Analysis Year				Percent Unopp	osed Left T	ums		0			0			0	<u> </u>		0		
Jurisdiction				Heaviest Lane	Volume, vel	h/h		0	500	0	0	500	0	0	0	0	0	0	0
				Start-Up Lost T	ìme, s			2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
General				Extension of Ef	fective Gree	en,s		2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Number of Calculation Iter	ations	15		Walk Interval,	5				0.0			0.0			0.0]		0.0	
Critical Merge Gap, s 3.70			Pedestrian Clea	ar Interval, s				0.0			0.0			0.0]		0.0		
Stored Vehicle Lane Length, ft 25.0			Receiving Lan	Receiving Lanes							2			2			2		
Length of Detected Vehic	le, ft	17.0		Heavy Vehicle	Equivalency	y Facto	ог	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.0
Stored Heavy Vehicle Ler	igth, ft	45		Bus Blockage	Time, s					14.4			14.4			14.4			14.
Queue Length Percentile 50		-	Parking Maneu	iver Time, s			18		18	18		18	18]	18	18]	18	
cceleration Rate, ft/s2 3.50		Opposing Right	t-Turn Lane	Influer	nce														
Stop Threshold Speed, mi	/h	5.0		Ct							C								
Pedestrians Pushing Butto	n, prop	0.65		Segment						וה	Signal							_	
Speed Limit to Base FFS I	Ratio	0.90					EB	٧	NB	11	Exclusi	ve Pe	destriar	1 Phase	e Time	, S		0.0)
Sneakers per Cycle, veh		2.0		Upstream Widt	h, ft	50		50							1.1	8			
Platoon Minimum Headwa	y,s∕veh	1.50		Restrictive Med	dian, ft	0		0		Left-Turn Equivalency Factor					1.0)5			
Platoon Maximum Headwa	∋y, s/veh	3.60		Right-Hand Cu	њ. %	70		70		Deceleration Rate, ft/s2 4.0)0			
Platoon Dispersion Factor		0.138		Right-Hand Ac	cess Points	4		4		1 I	Critical	Heady	vay (pe	mitted	left tu	m), s		4.5	5
Demand Growth, %	+ -	0	÷	Mid-Segment D)elay, s/veh	0.0		0.0]	Follow	Up He	adway	(permi	tte <mark>d l</mark> ef	t tum),	5	2.5	5
Access Point			Ac	cess Points															
Critical Headway (left from	n major), s 4.	.1	A A	ctive		4.						٦.					_	D 1 -	
Follow-Up Headway (left	from major), s 2.	.2	PI	HF 1.00		1>				•	-	_	unt: 2		Ne			Delete	
Right-Turn Equivalency F	actor 2	.20				EBT	EBR	WB			WBR	NBL			IBR	SBL	SB		SBR
Maximum Turn Bay Lengt	h,ft 2	50	D	emand, veh/h		050	100	80	10		00	80	0	1(80	0	10)0
Deceleration Rate, ft/s2	6	.7	تا	anes	0 2		0	0	2	0		1	0	1		1	0	1	
Right-Turn Speed, ft/s	2	n	N N	ame							Loca	tion, ft	600						

10. Run Full Optimization

Full Optimization	
 ◇ Input Parameters Global Optimization Objective Function Overall Delay Cycle Length Splits Y Phasing Sequence Pallas Phasing 	Start
 Optimization Status Diagnostic Messages 	Info Stop

11. **Optimization** Results

Full Optimization				Þ
 Optimization Stat 	tus			
	Overa	II Delay		
	Original	263.7 sec/veh		
	Optimum	169.1 sec/veh		
	Average	175.0 sec/veh		
	Improvement	35.9%		
	Run	Status		
	Generation Number	200 out of 200		
	Generation Optimum	101		
	Total Time Elapsed	43 sec		
	Termination via max n	umber of generations	s	
Diagnostic Messa	ages			
			s	itart
No messages	to report at this time.			nfo
		Save	Cancel	

12. View/Print **Results** Summary Report

		HCS 2	J10 S	ignali	zed li	nterse	ection	I Kes	sults 3	umm	ary				
General Inform	nation								Intersec	tion Inf	ormatio	n			
Agency	lation	GDOT							Duration		0.25		-	116	
Analyst		Design Engineer	_	Analys	is Data	Apr 1	2 2012		Area Typ						
Jurisdiction		Clayton County		Analysis Date Apr 12, 2012 Time Period 2032 PM					PHE	/e	Other		1 + 1 1 + 1		÷
Intersection		SR 42 @ Forest Pk	MA/		is Year		1111		Analysis	Period	1> 7:0	00	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		-
File Name		pdt example.xus	,	[/ dialiya	10 1001	LUIL			Analysis	renoa	10-1.4			S 1. S 1.	
Project Descrip	tion	Existing Geometry I	Design	VeerTe									-1 🖣	<u>)''</u>	
Project Descrip	uon	Existing Geometry (Jesign	rearin	anic										
Demand Infor	mation				EB			W	в		NB			SB	
Approach Move	ement			L	T	R	L	Т		L	T	R	L	Т	R
Demand (v), ve	eh/h			350	1315	285	385	74	0 125	130	320	240	225	780	31
Signal Informa			-										- 1 -		
Cycle, s	110.0		2			1	1					4	Y	2	4
Offset, s	0	Reference Point	End	Green		0.0	0.0	0.0		0.0					Б
Uncoordinated	No	Simult. Gap E/W	On	Yellow		0.0	0.0	0.0		0.0		4	P _	∠	
Force Mode	Fixed	Simult. Gap N/S	On	Red	0.0	0.0	0.0	0.0	0.0	0.0	_	2		7	
Timer Results			EBL		EBT	WB	L	WBT	NB		NBT	SB		SBT	
Assigned Phase			7		4	3	-	8			2			6	
Case Number			2.0	-	4.0	2.0		4.0		-	5.0			5.0	
Phase Duration, s				32.2	, ,	50.0	21.0	,	38.8		39.0				39.0
Change Period		5		5.0		5.0	5.0	-	5.0		-	5.0			5.0
Max Allow Hea			_	0.0		0.0	0.0		0.0			0.0			0.0
Queue Clearan				0.0	-	0.0	0.0	-	0.0		+	0.0		-	0.0
Green Extensio				0.0		0.0	0.0		0.0			0.0			0.0
Phase Call Pro		(9%) 5	_	0.00 0.00 0.00		,	0.00		0.00				0.00		
Max Out Proba			_	0.00 0.00 0.00		_	0.00		0.00			_	0.00		
Movement Gro Approach Move		suits		L I	EB T	R	L	WB	R	L	NB	R	L	SB	R
Approach Move Assigned Move				7	4	14	3	8	18	5	2	12	1	6	16
Assigned Move Adjusted Flow		unda da	_	0	4	14	3	8 0	18	0	2	12	0	0	10
				0	0	0	0	0	0	0	0	0	0	0	0
		ow Rate (s), veh/h/ln		-	0.0	0.0	-	-		0.0	0.0	0.0	0.0	0.0	0
Queue Service				0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Cycle Queue C Capacity (c), ve		e nime (g.), s	_	412	714	0.0 670	238	527	501	0.0	0.0 518	439	188	539	45
		tie 00		0.924	1.233	1.282					0.672	439	1.303	1.572	
Volume-to-Cap Available Capa				0.924	1.233	1.282	1.758 0	0.915	0.915	2.159	0.672	0.594	1.303	1.5/2	0.7
		, ven/n h/In (95th percentile)		15.2	58.8	61.9	47.3	21.1	-	21.8	13.5	10.4	22.9	82.4	13.
Back of Queue Overflow Queu				0.0	0.0	0.0	47.3	21.1	20.3	21.8	0.0	0.0	0.0	82.4	13
				1.17	0.00	0.00	4.25	0.00		3.09	0.00	0.0	3.09	0.00	1.0
		RQ) (95th percentile))		32.5	32.5		38.7		3.09	33.1	32.2	50.4	38.0	1.0
Uniform Delay Incremental De				40.4	32.5	32.5	47.0 357.7	38.7		568.5	33.1 6.8	32.2 5.8	50.4 169.7	266.4	34
					116.9	138.0	357.7	20.2		0.0	0.0	5.8 0.0	169.7	208.4	10.
	nitial Queue Delay (dz), s/veh			0.0	149.4		404.7	58.9		623.5	39.9	38.0	220.1	304.4	44
Initial Queue D	ontrol Delay (d), s/veh					170.5									
Initial Queue D Control Delay (
Initial Queue D	e (LOS)			D 139	F	F	F 164	E	E	F 149	D	D F	F 228	F	E

13. View Messages Report

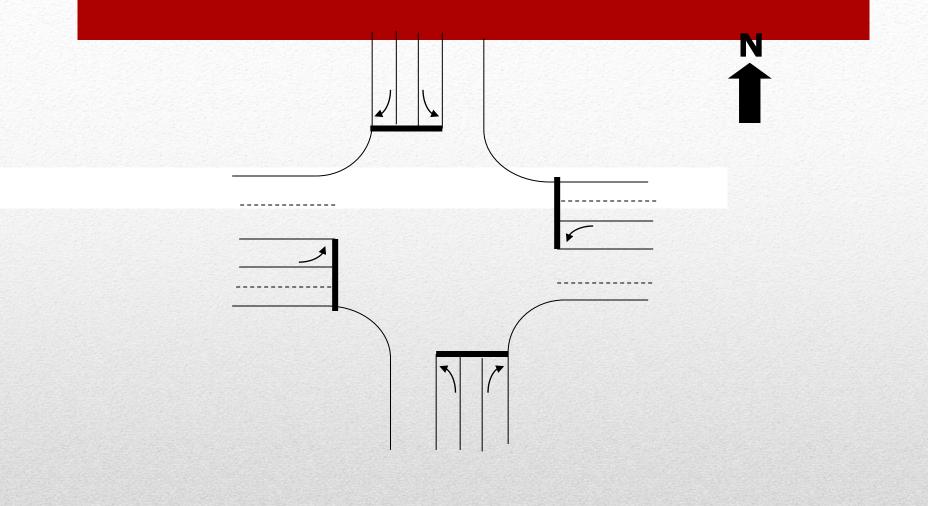
- Look for any warnings

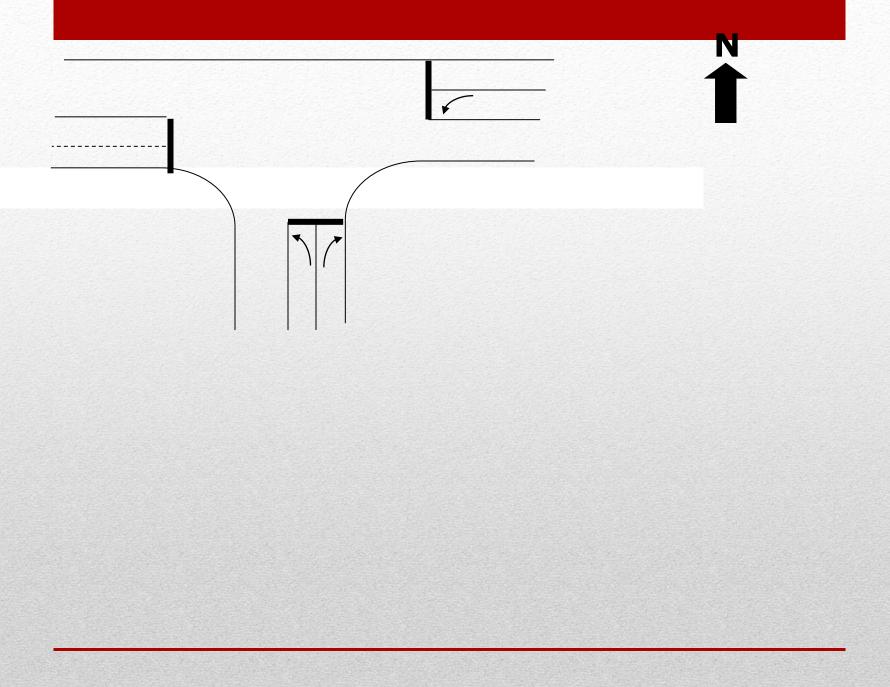
--- Messages ----

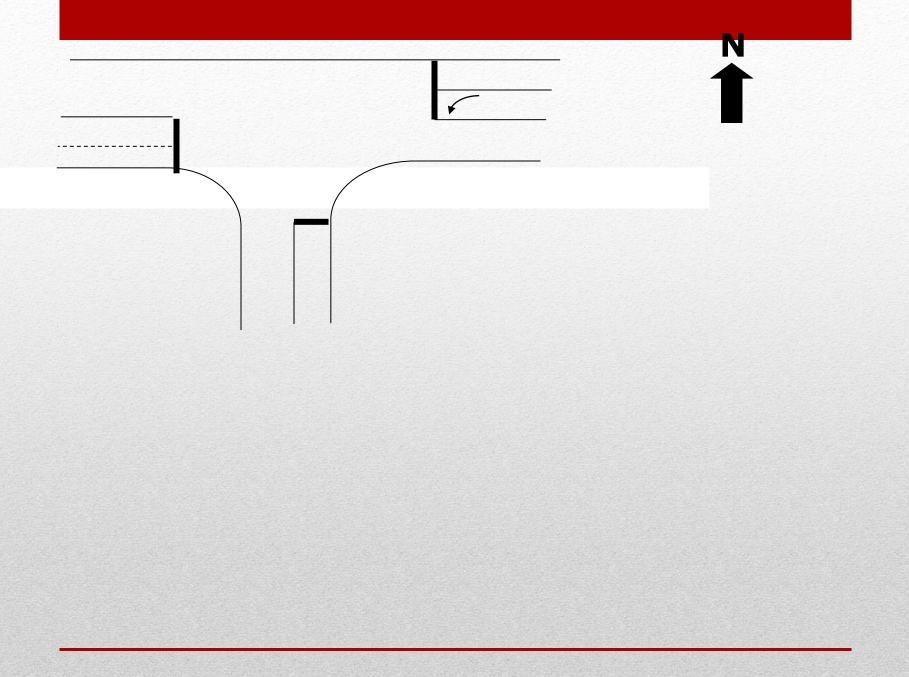
100404007040

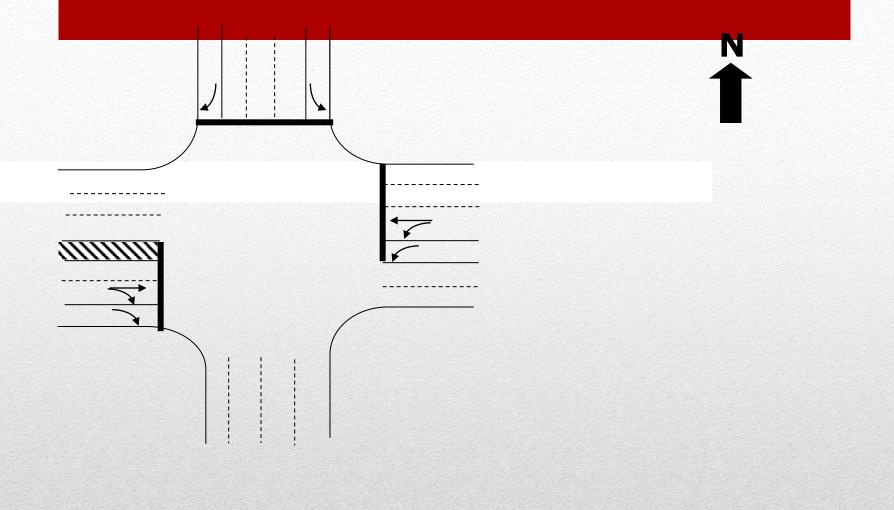
WARNING: Since queue spillover from turn lanes and spillback into upstream intersections is not accounted for in the HCM procedures, use of a simulation tool may be advised in situations where the Queue Storage Ratio exceeds 1.0.

--- Comments ---









RESULTS