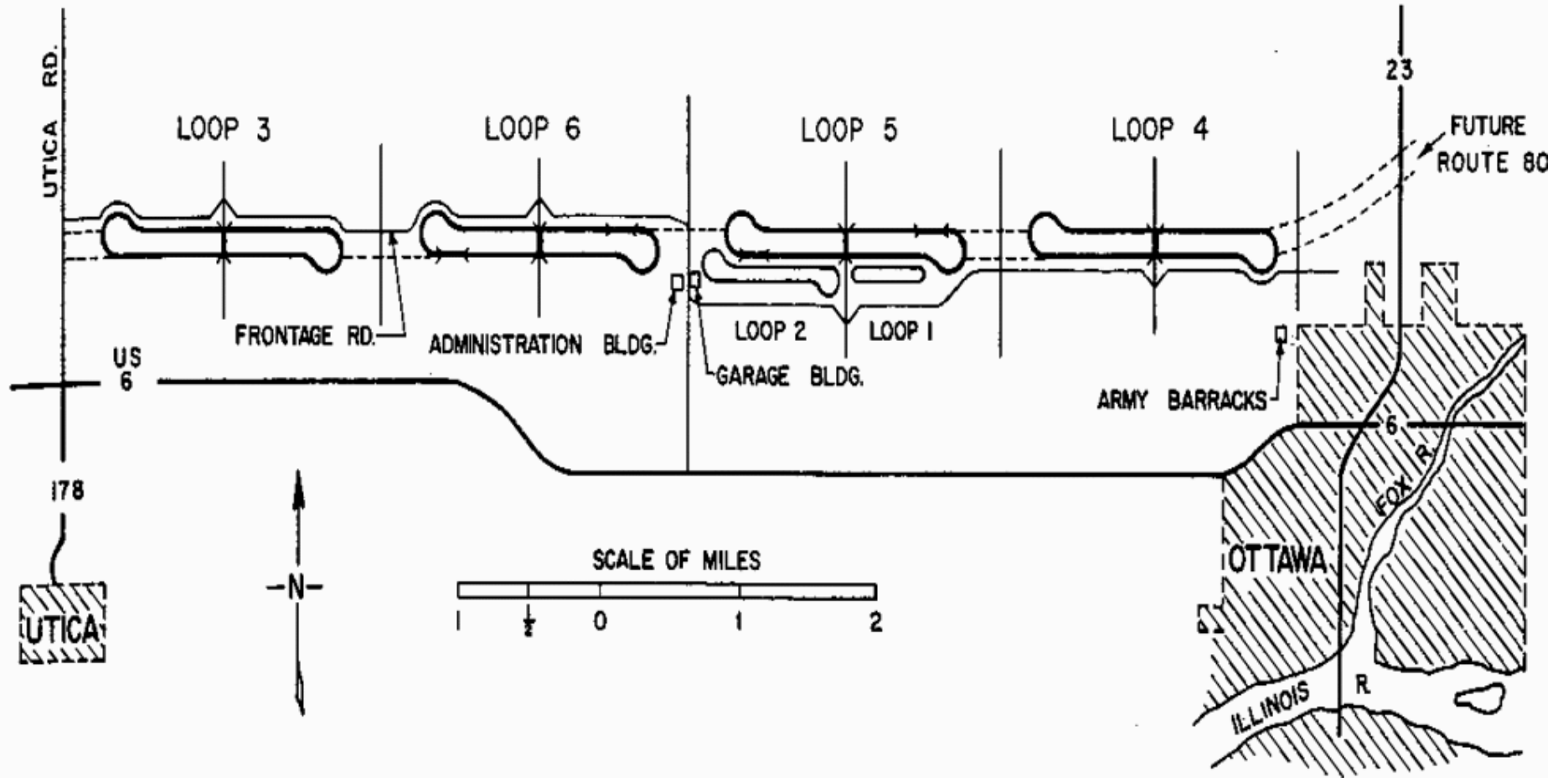


Pavement Performance

AASHO Road Test



AASHO Road Test



Source: <http://www.fhwa.dot.gov>

AASHO Road Test

Table 5: AASHO Road Test Structural Design Thickness Summary

Loop No.	AC Thickness (in.)	Base Thickness (in.)	Subbase Thickness (in.)
1	1.0	0.0	0.0
	3.0	6.0	8.0
	5.0	--	16.0
2	1.0	0.0	0.0
	2.0	3.0	4.0
	3.0	6.0	8.0
3	2.0	0.0	0.0
	3.0	3.0	4.0
	4.0	6.0	--
4	3.0	0.0	4.0
	4.0	3.0	8.0
	5.0	6.0	12.0
5	3.0	3.0	4.0
	4.0	6.0	8.0
	5.0	9.0	12.0
6	4.0	3.0	8.0
	5.0	6.0	12.0
	6.0	9.0	16.0

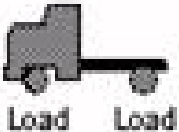
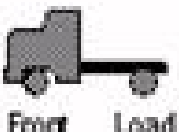


AASHO Road Test

Table 13: Rigid Pavement Structural Design Parameters





Loop No.	PCC Slab Thickness (in.)	Subbase Thickness (in.)	Transverse Dowel Bars (Diameter x Length)
1	2.5 5.0 9.5 12.5	0, 6.0 0, 6.0 0, 6.0 0, 6.0	3/8" x 12" 5/8" x 12" 1 1/4" x 18" 1 5/8" x 18"
2	2.5 3.5 5.0	0, 3.0, 6.0 0, 3.0, 6.0 0, 3.0, 6.0	3/8" x 12" 1/2" x 12" 5/8" x 12"
3	3.5 5.0 6.5 8.0	3.0, 6.0, 9.0 3.0, 6.0, 9.0 3.0, 6.0, 9.0 3.0, 6.0, 9.0	1/2" x 12" 5/8" x 12" 7/8" x 18" 1" x 18"
4	5.0 6.5 8.0 9.5	3.0, 6.0, 9.0 3.0, 6.0, 9.0 3.0, 6.0, 9.0 3.0, 6.0, 9.0	5/8" x 12" 7/8" x 18" 1" x 18" 1 1/4" x 18"
5	6.5 8.0 9.5 11.0	3.0, 6.0, 9.0 3.0, 6.0, 9.0 3.0, 6.0, 9.0 3.0, 6.0, 9.0	7/8" x 18" 1" x 18" 1 1/4" x 18" 1 3/8" x 18"
6	8.0 9.5 11.0 12.5	3.0, 6.0, 9.0 3.0, 6.0, 9.0 3.0, 6.0, 9.0 3.0, 6.0, 9.0	1" x 18" 1 1/4" x 18" 1 3/8" x 18" 1 5/8" x 18"

Source: WSDOT Pavement Guide Interactive CD-ROM



AASHO Road Test

		Weight in Kips		
		Front Axle	Load Axle	Gross Weight
②	① 	2	2	4
	② 	2	6	8
③	① 	4	12	28
	② 	6	24	54

AASHO Road Test

		Weight in Kips		
		Front Axle	Load Axle	Gross Weight
④	① 	6	18	42
	② 	9	32	73
⑤	① 	6	22.4	50.8
	② 	9	40	89

AASHO Road Test

		Weight in Kips		
		Front Axle	Load Axle	Gross Weight
⑥	① 	9	30	69
	② 	12	48	108

Present Serviceability Rating

“The judgment of an observer as to the current ability of a pavement to serve the traffic it is meant to serve”

Present Serviceability Rating

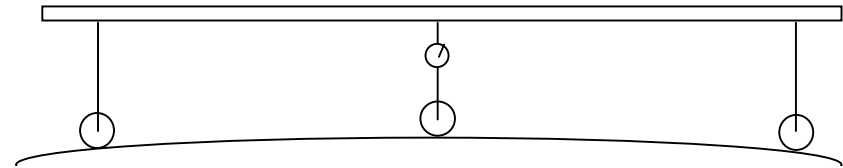
Acceptable?		5	—	Very Good
		4	—	Good
Yes	<input type="checkbox"/>	3	—	Fair
No	<input type="checkbox"/>	2	—	Poor
Undecided	<input type="checkbox"/>	1	—	Very Poor
		0	—	

Section Identification _____ Rating _____
Rater _____ Date _____ Time _____ Vehicle _____

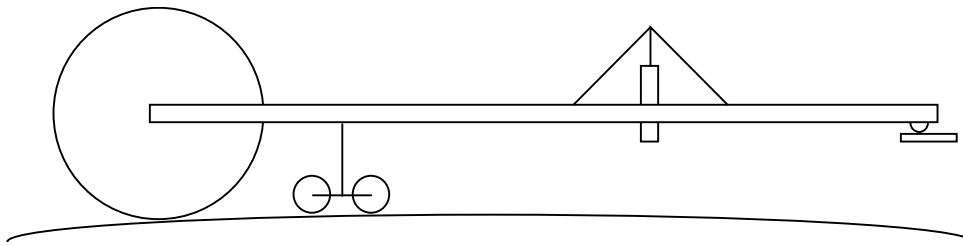
Acceptable Ride Quality

PSR	Acceptable?
3.0	88%
2.5	45%
2.0	15%

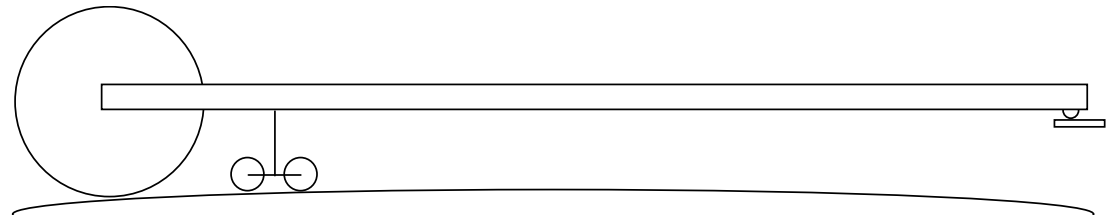
Measuring Roughness



Straightedge



Slope Profilometer



CHLOE Profilometer

CHLOE Profilometer



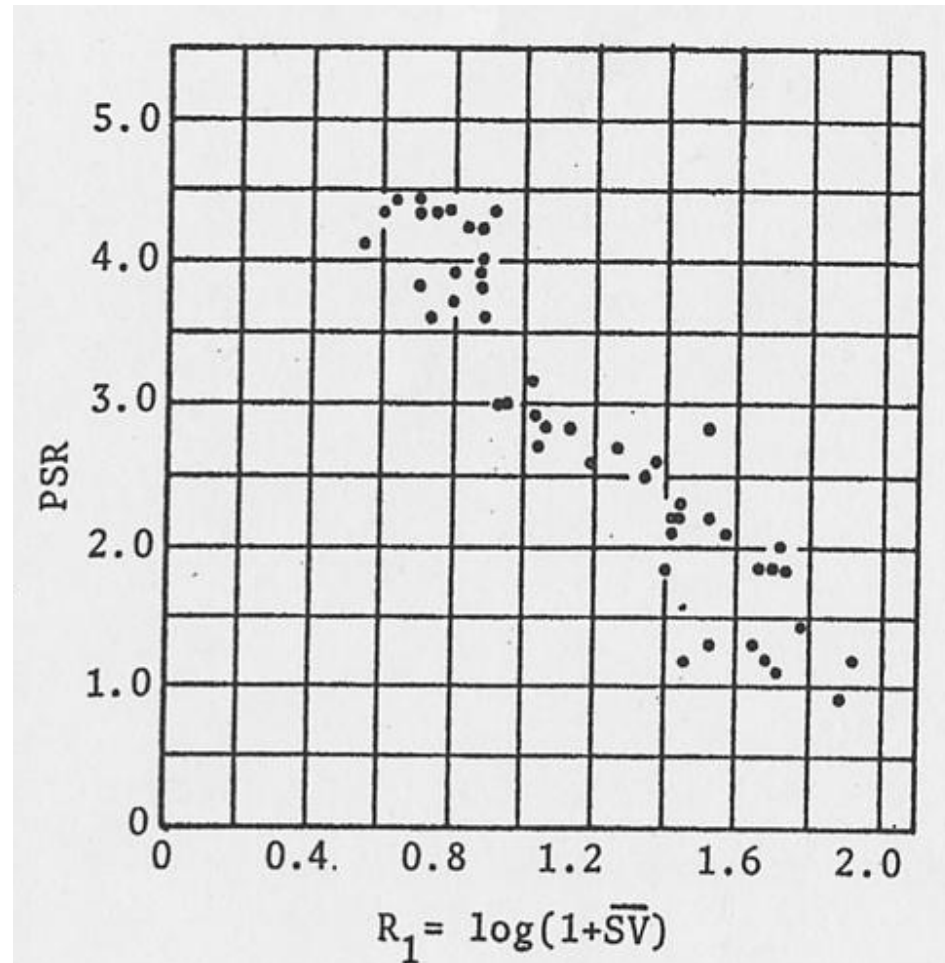
Source: State Library and Archives of Florida

Slope Variance

$$SV = \frac{\sum S^2 - \frac{1}{n} (\sum S)^2}{n-1} \times 10^6$$

S = slope (radians)

Flexible Pavements

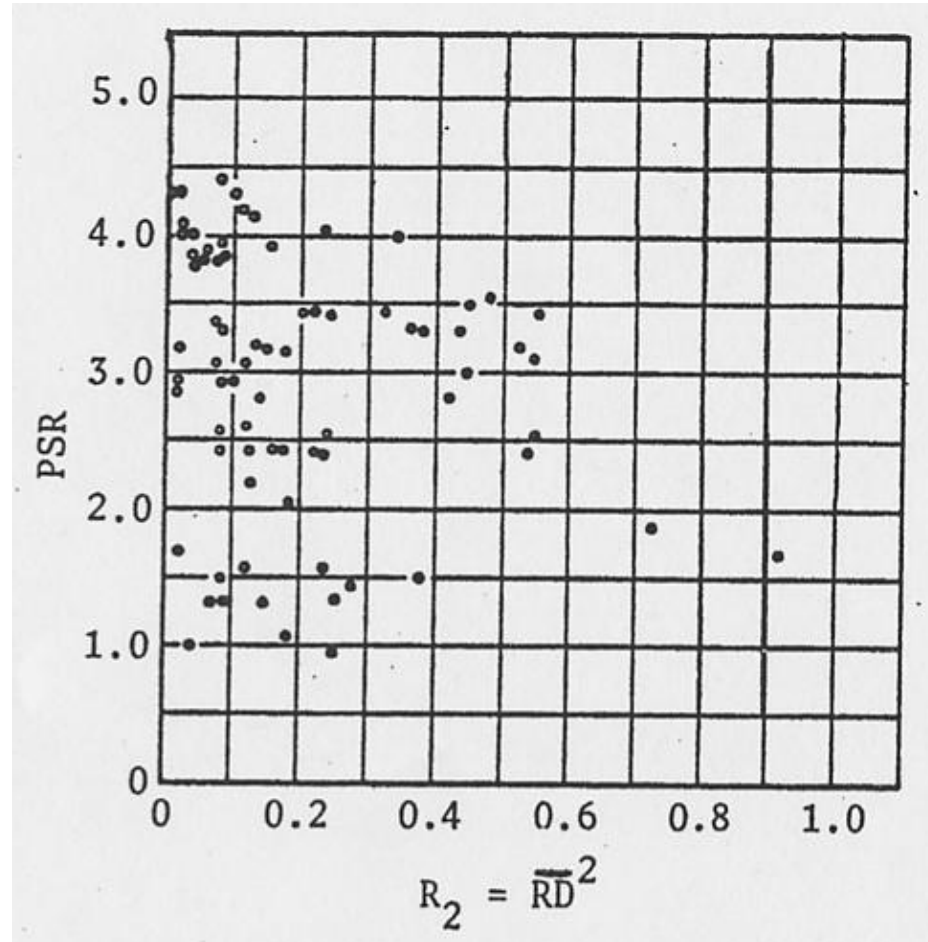


Rut Depth



Source: www.halifax.ca/Works/AsphaltDefectGuide.html

Flexible Pavements



Cracking

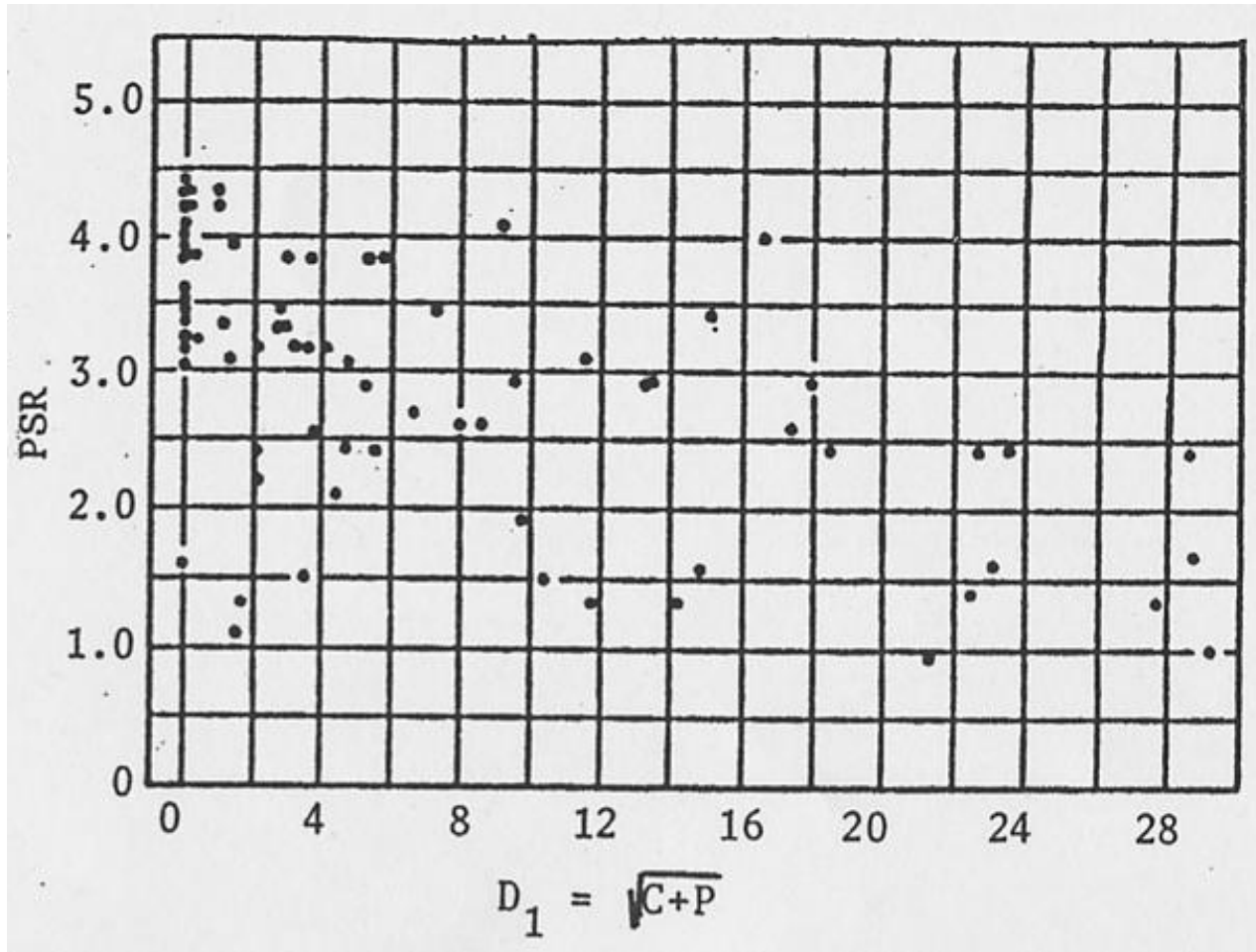


Patching

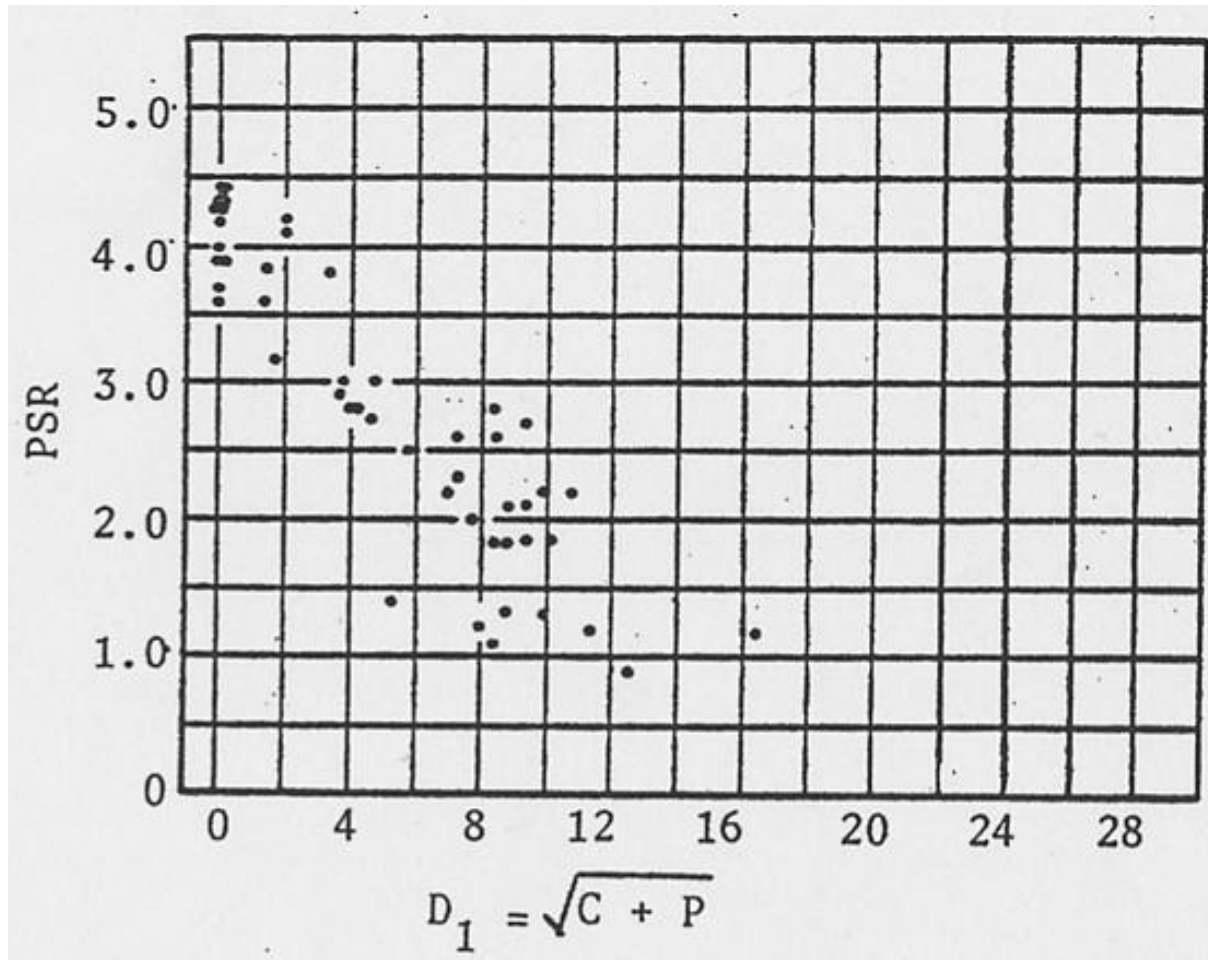


Source: www.asphaltrescue.com

Flexible Pavements



Rigid Pavements



Present Serviceability Index

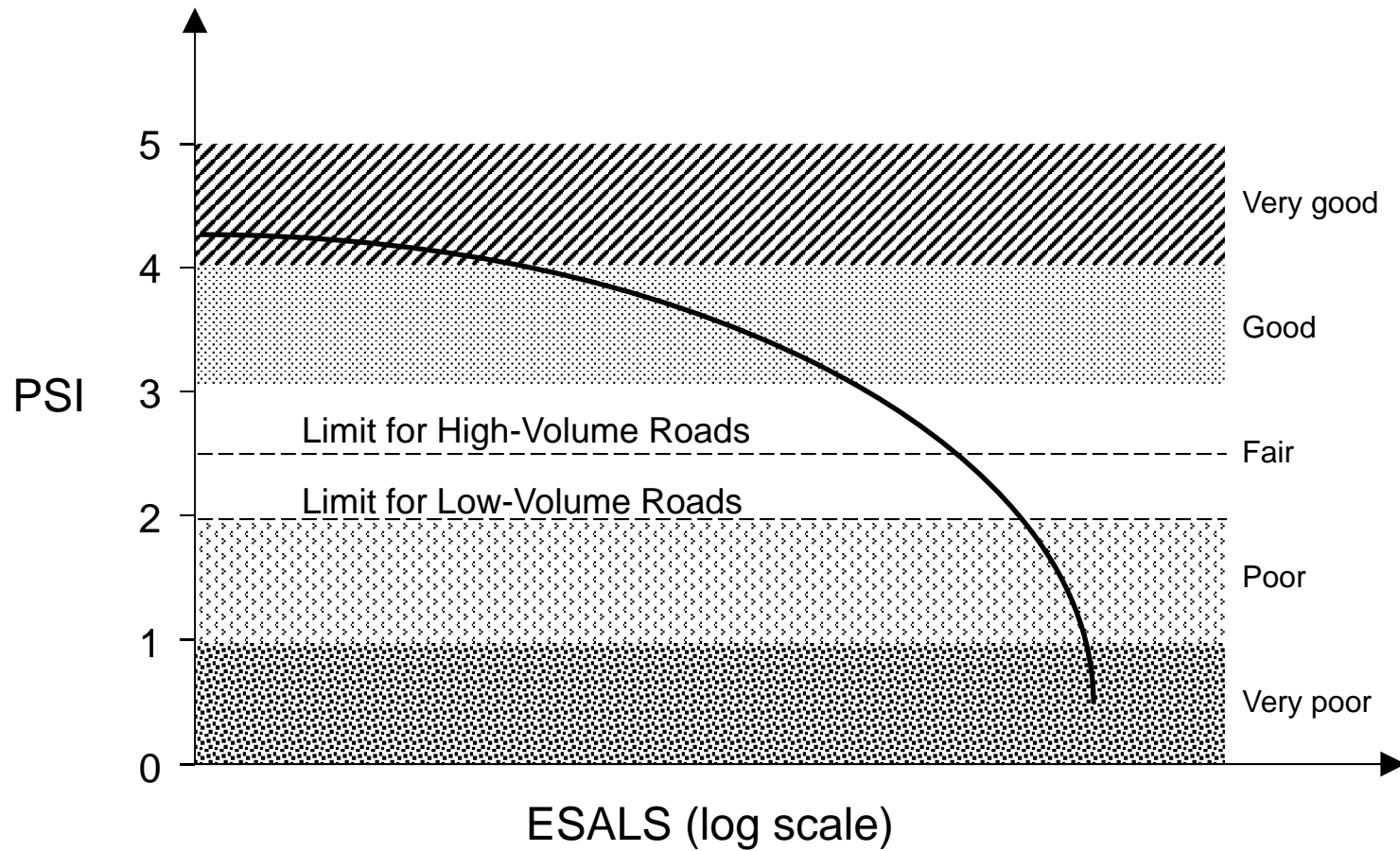
Flexible Pavements

$$PSI = 5.03 - 1.9 \log(1 + SV) - 1.38RD^2 - 0.01\sqrt{C + P}$$

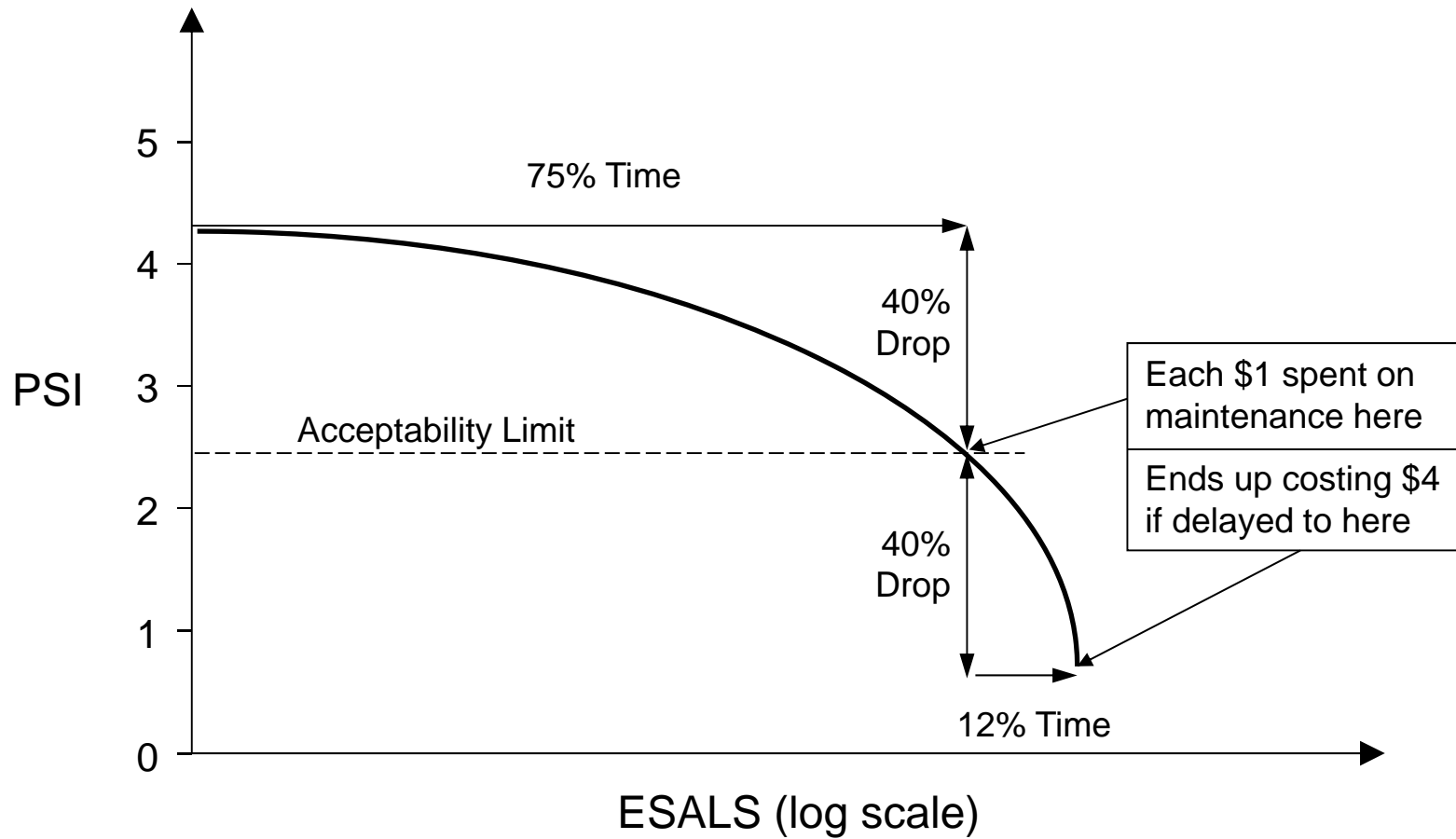
Rigid Pavements

$$PSI = 5.41 - 1.8 \log(1 + SV) - 0.09\sqrt{C + P}$$

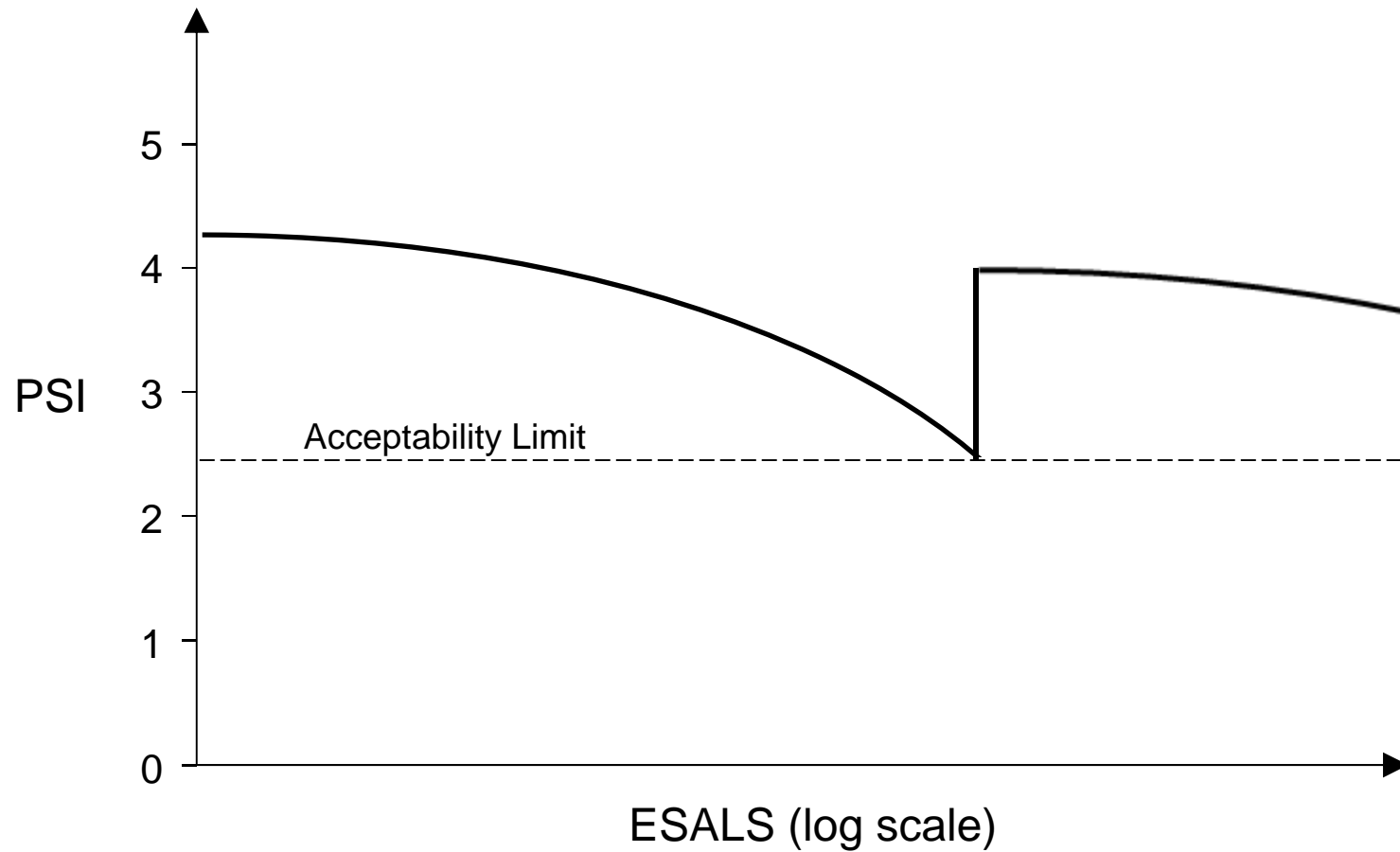
Pavement Performance



Pavement Performance



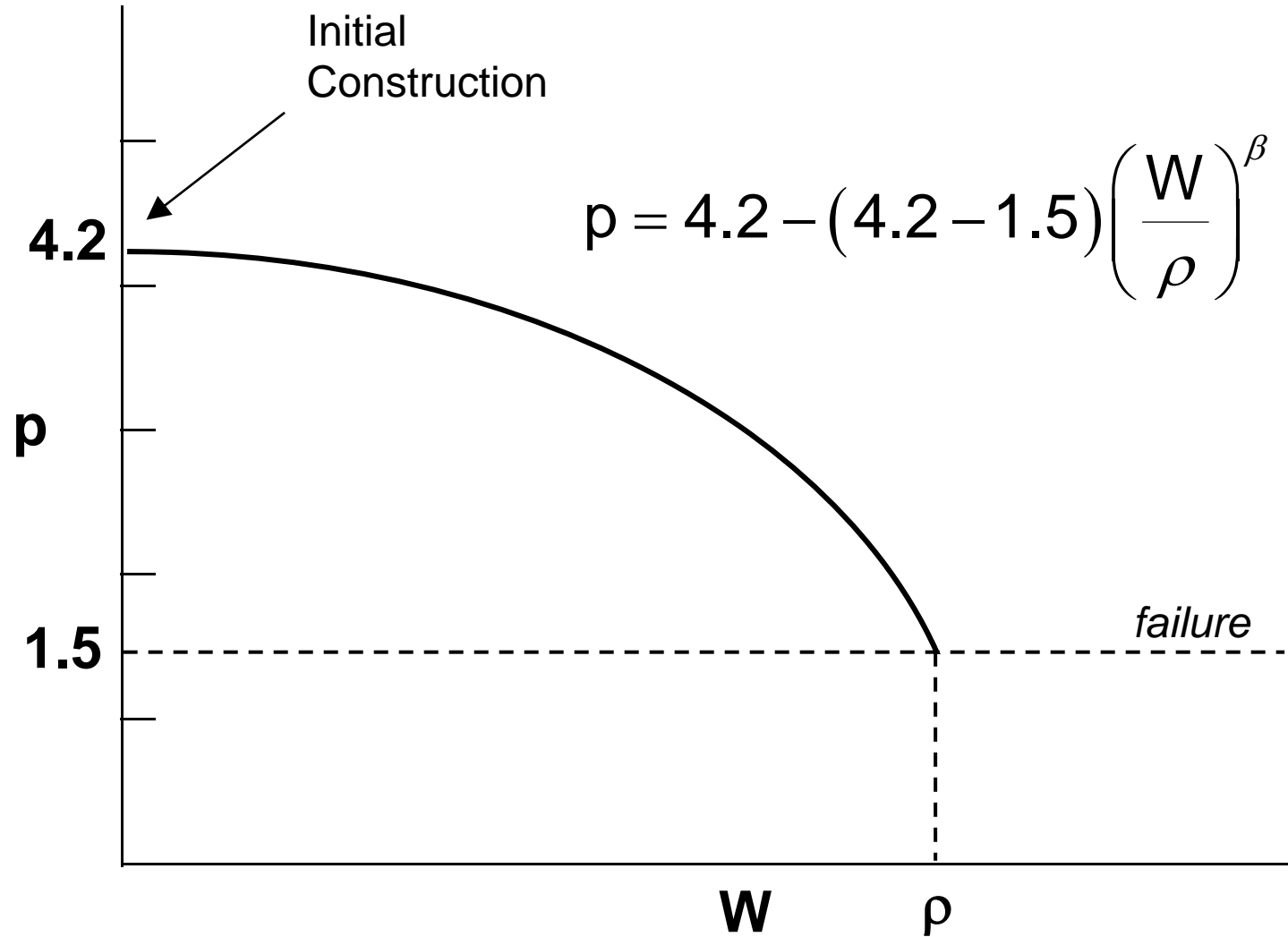
Pavement Performance



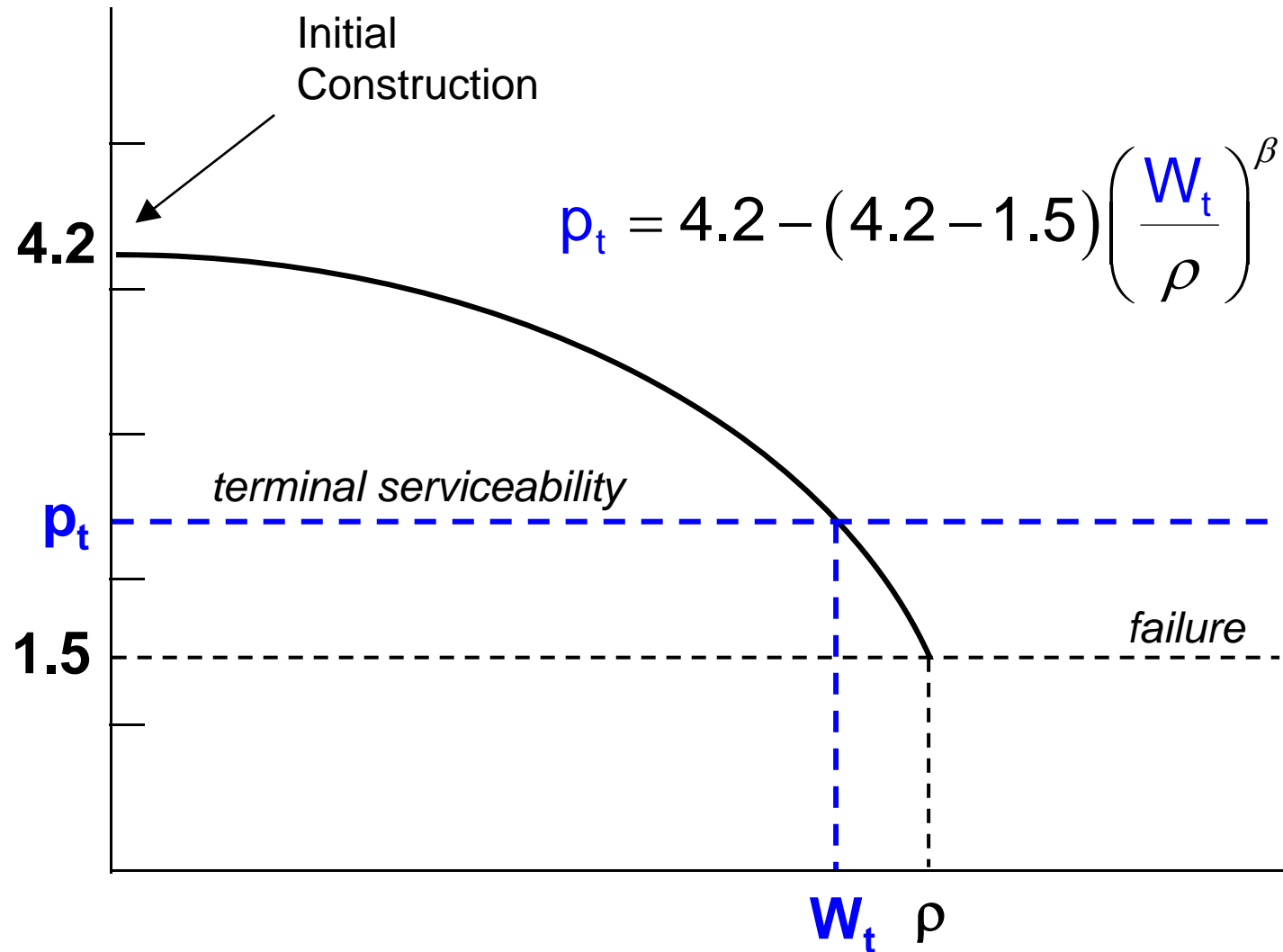
AASHTO Design Equation

(Flexible Pavements)

Pavement Performance



Pavement Performance

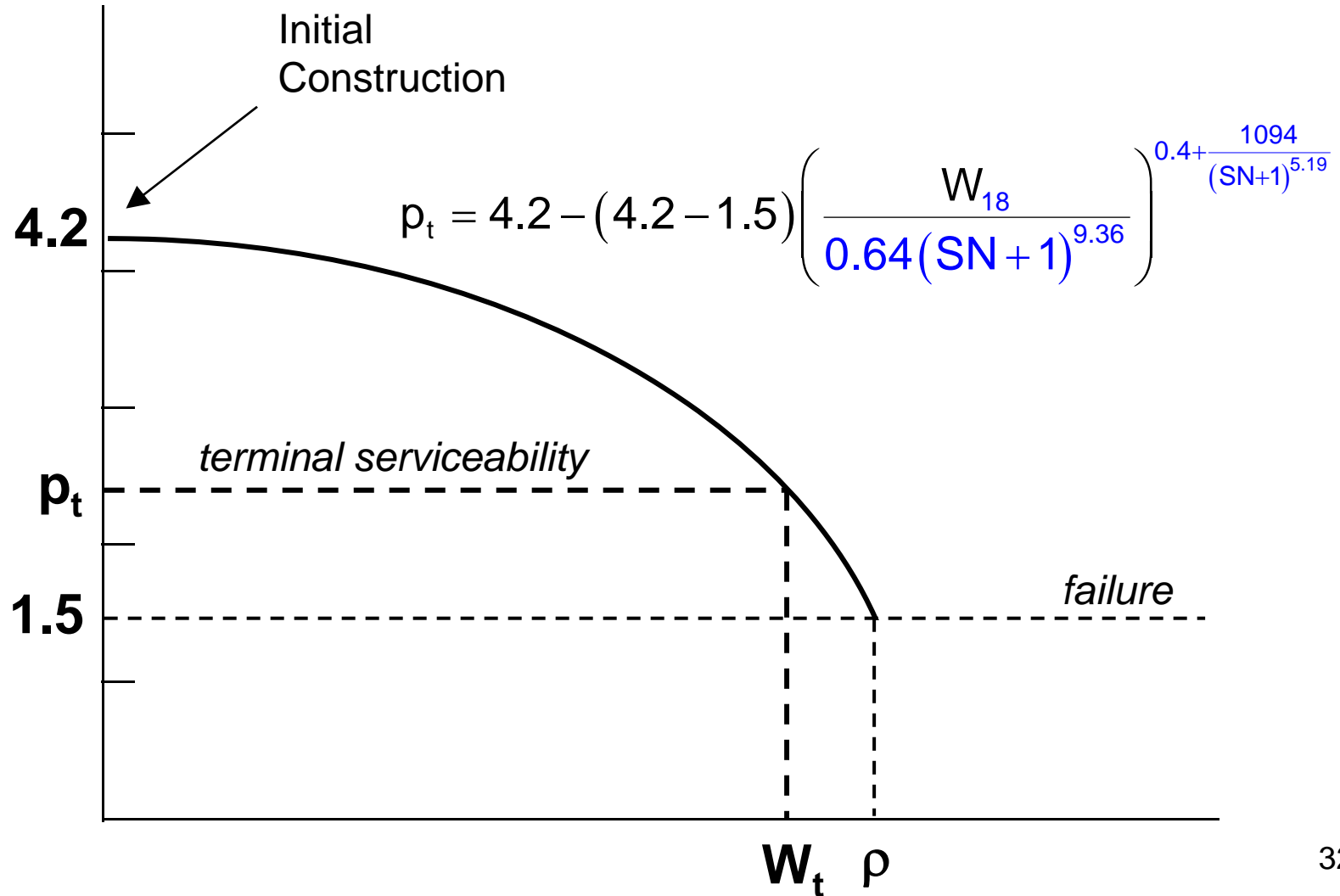


Flexible Performance

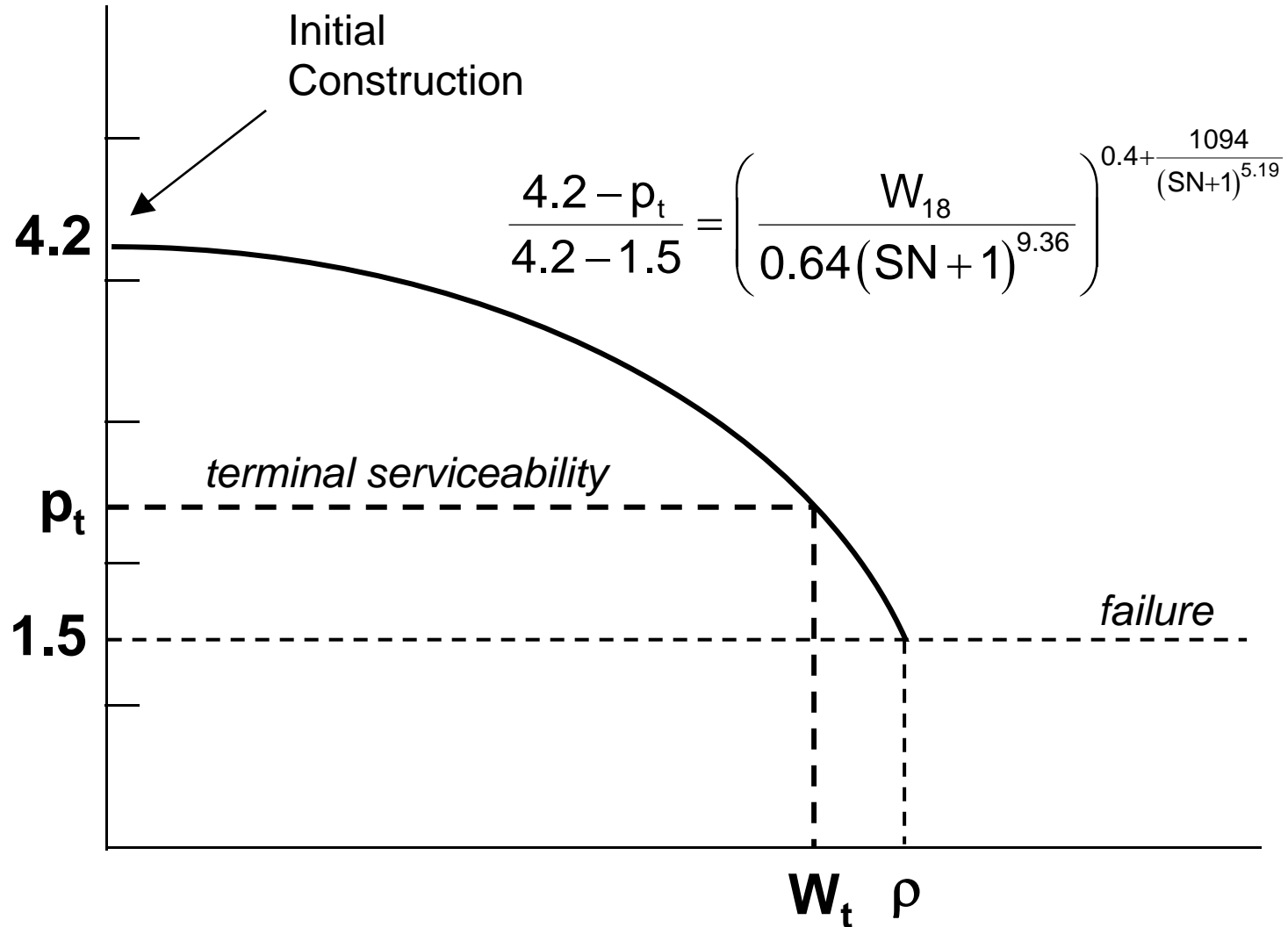
$$\beta_{18} = 0.4 + \frac{0.081(18+1)^{3.23}}{(SN+1)^{5.19} 1^{3.23}} = 0.4 + \frac{1094}{(SN+1)^{5.19}}$$

$$\rho_{18} = \frac{10^{5.93} (SN+1)^{9.36} 1^{4.33}}{(18+1)^{4.79}} = 0.64(SN+1)^{9.36}$$

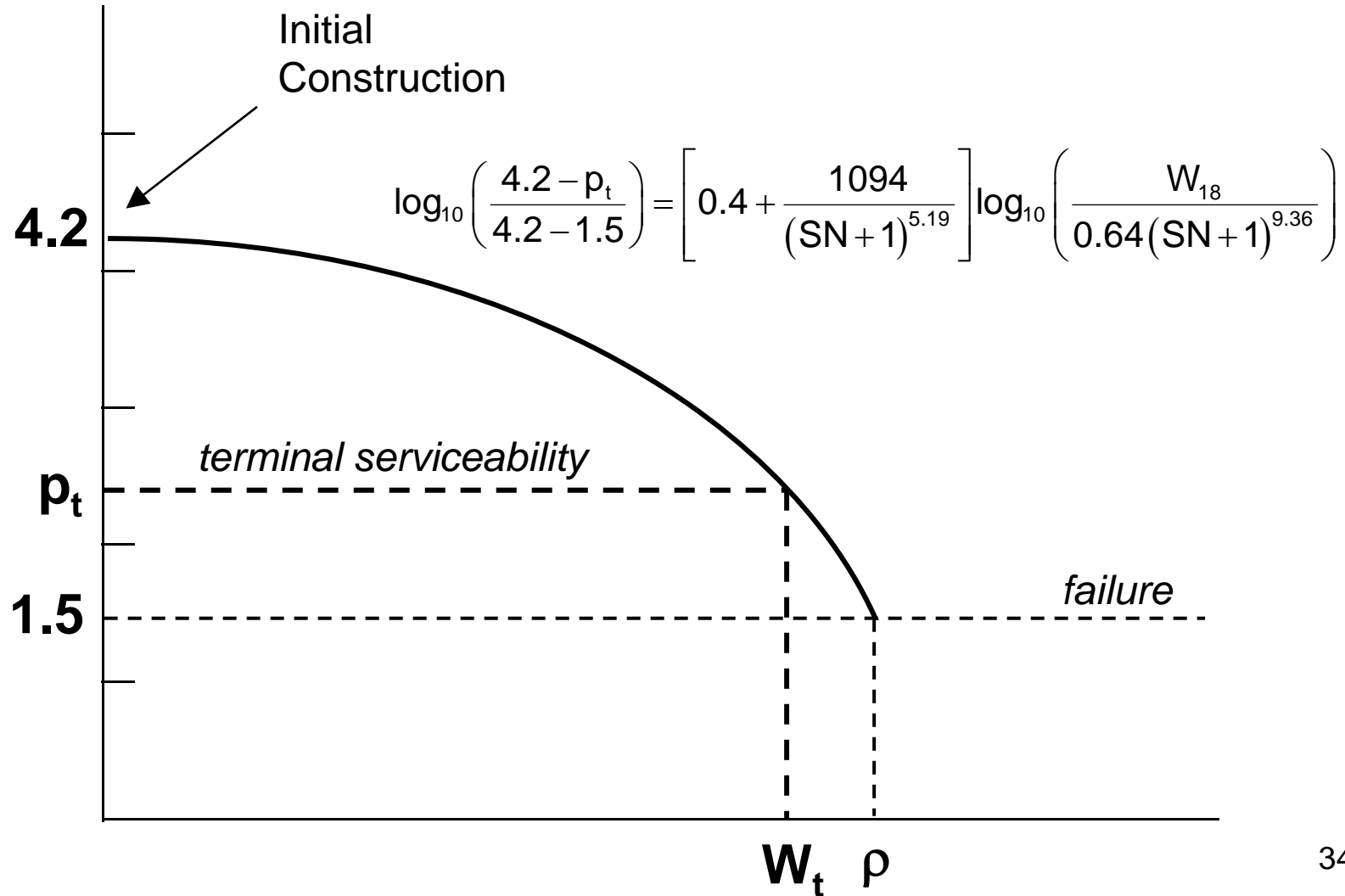
Pavement Performance



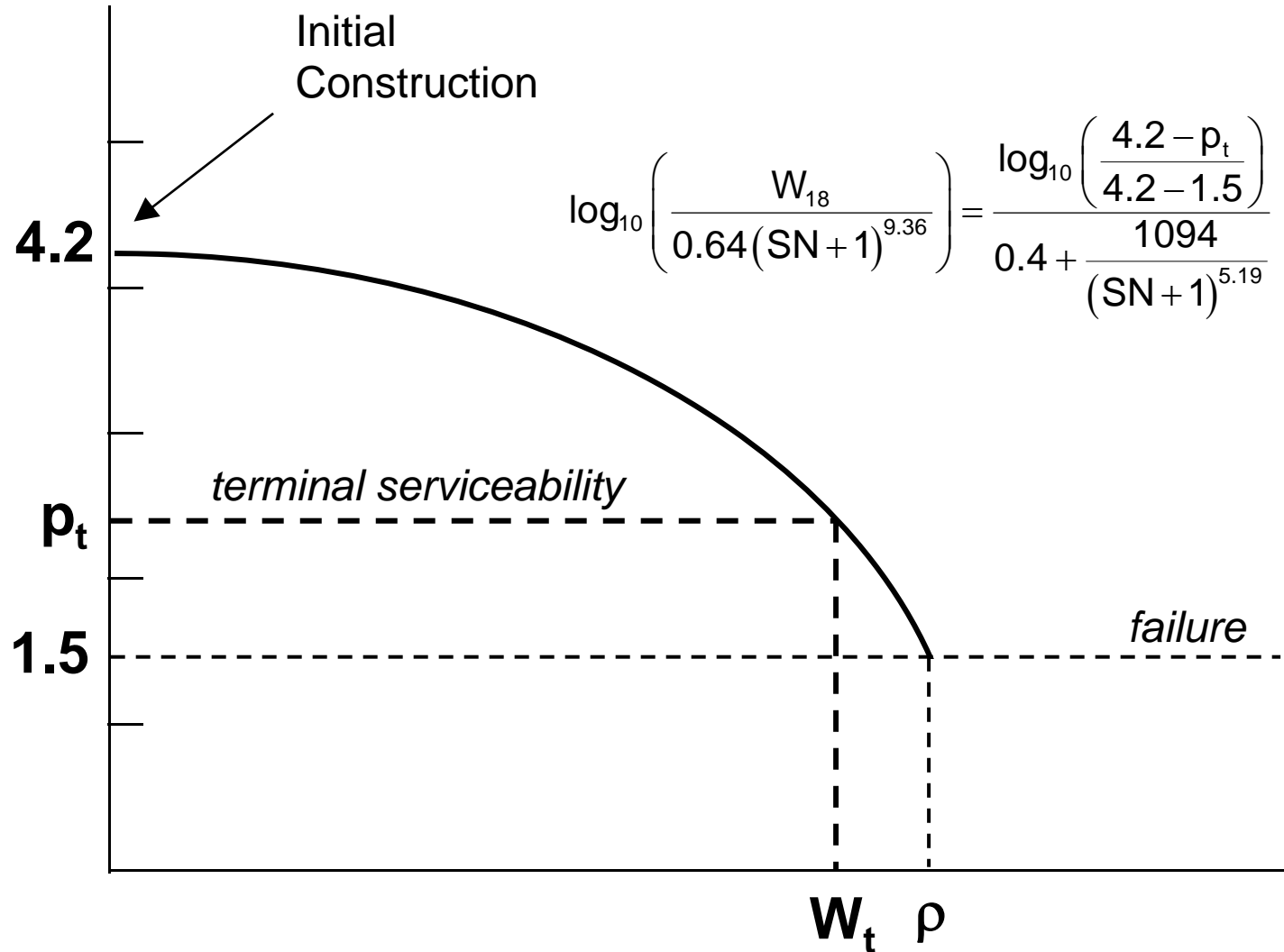
Pavement Performance



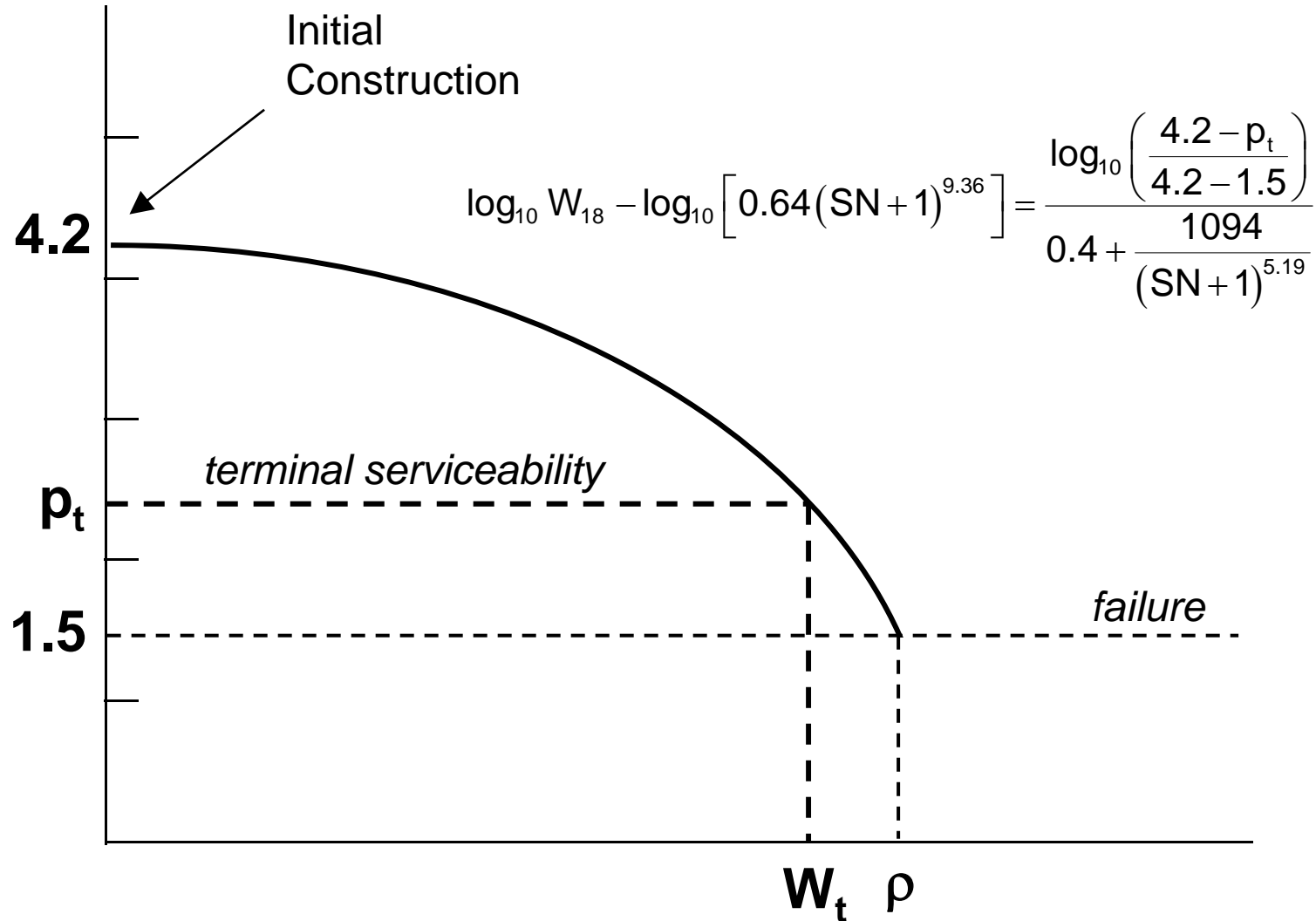
Pavement Performance



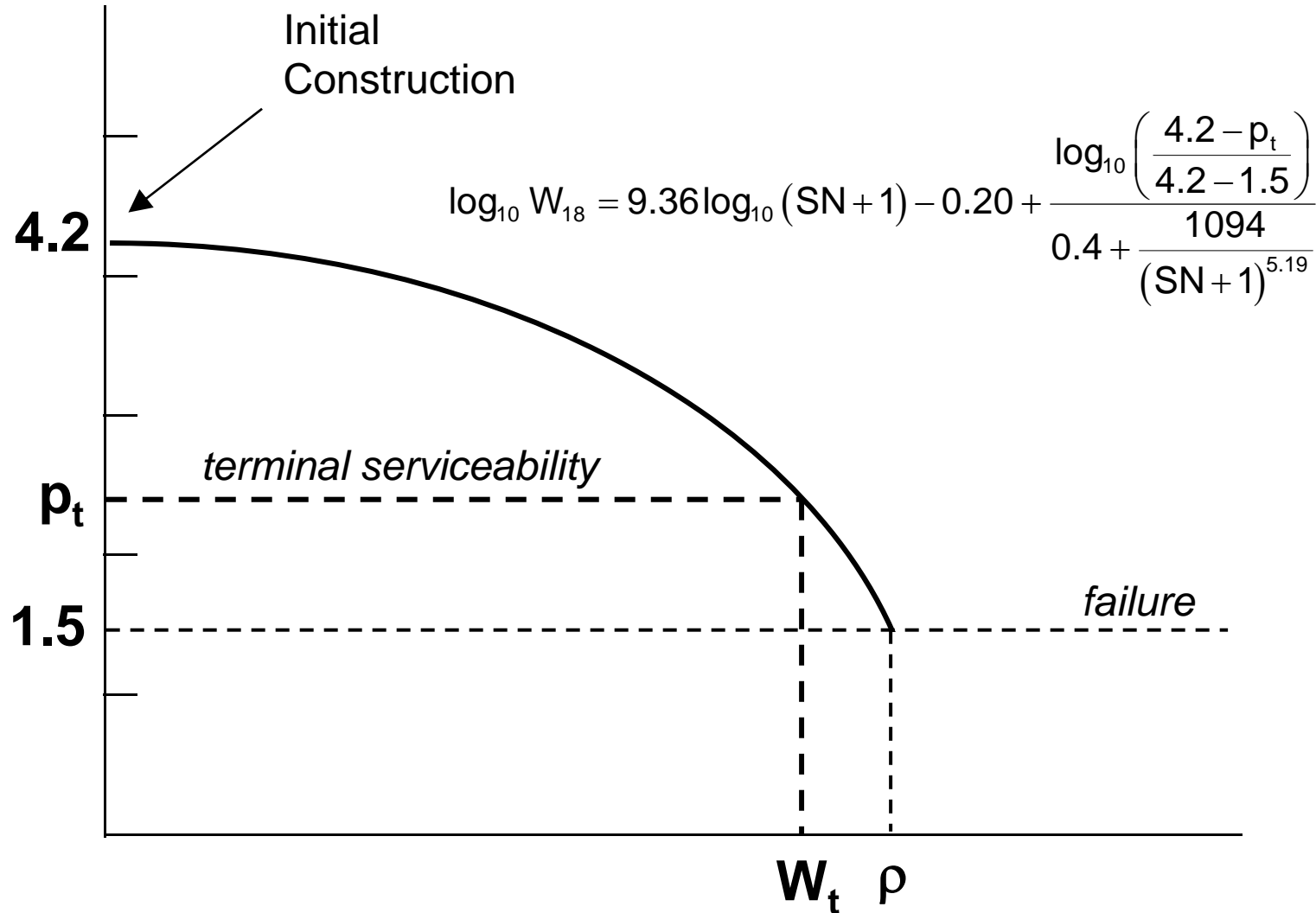
Pavement Performance



Pavement Performance



Pavement Performance



Flexible Performance

$$\log_{10} W_{18} = 9.36 \log_{10} (\text{SN} + 1) - 0.20 + \frac{\log_{10} \left(\frac{4.2 - p_t}{4.2 - 1.5} \right)}{0.4 + \frac{1094}{(\text{SN} + 1)^{5.19}}}$$

1966

AASHO Road Test

Climate

Average Mean Temperature (July)	24.5°C (76°F)
Average Mean Temperature (January)	-2.8°C (27°F)
Annual Average Rainfall	837 mm (34 in)
Average Depth of Frost (for fine-grained soil)	711 mm (28 in)

AASHO Road Test

Subgrade

Soil Classification	A-6
Percent Passing a #200 Sieve	82%
Liquid Limit	31%
Plasticity Index	16%
Average Degree of Saturation	85%
Average CBR	2.9

Flexible Performance

$$\log_{10} W_{18} = 9.36 \log_{10} (SN + 1) - 0.20 + \frac{\log_{10} \left(\frac{4.2 - p_t}{4.2 - 1.5} \right)}{0.4 + \frac{1094}{(SN + 1)^{5.19}}} + \log \frac{1}{R} + 0.372 (S_i - 3.0)$$

1972

Subgrade
Support
Factor



Regional
Climate
Factor



Regional Factor

Condition	R value
Roadbed materials frozen to a depth of 5 in. or more (winter)	0.2 – 1.0
Roadbed materials dry (summer and fall)	0.5 – 1.5
Roadbed materials wet (spring thaw)	4.0 – 5.0

Regional Factor

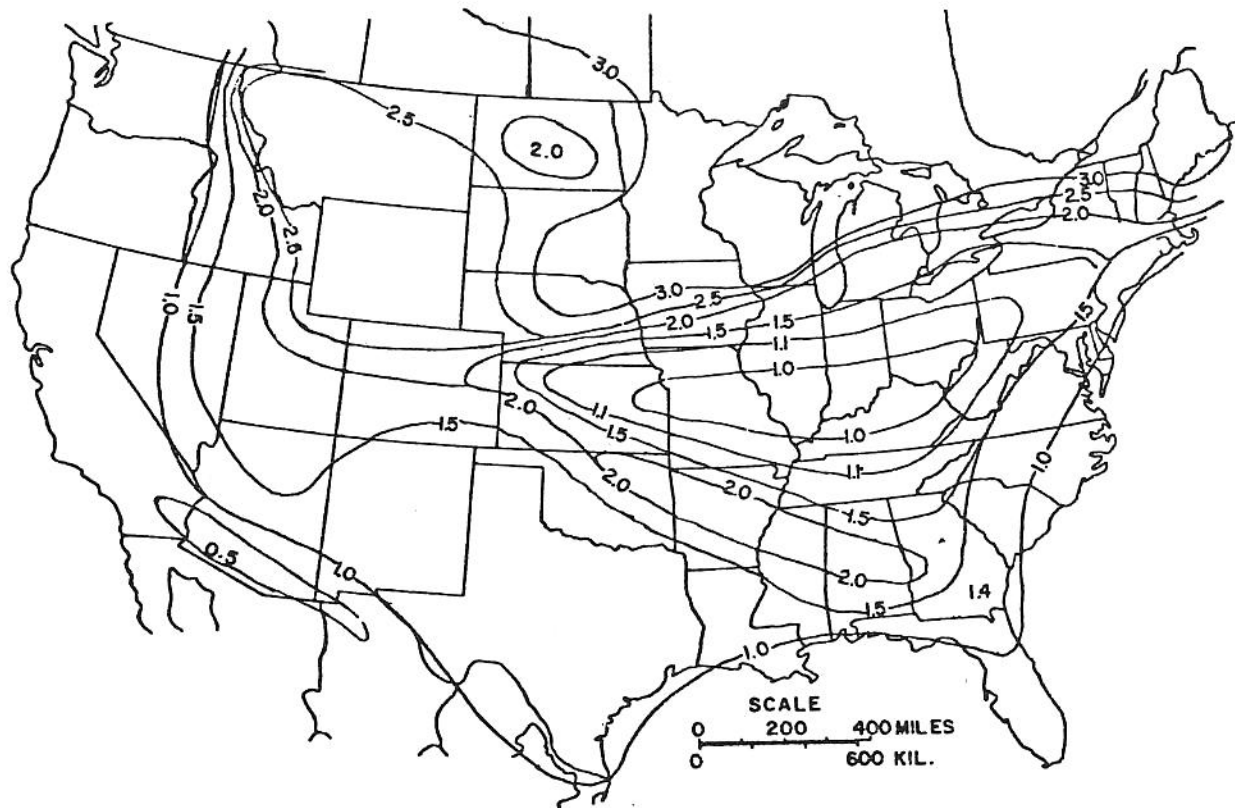
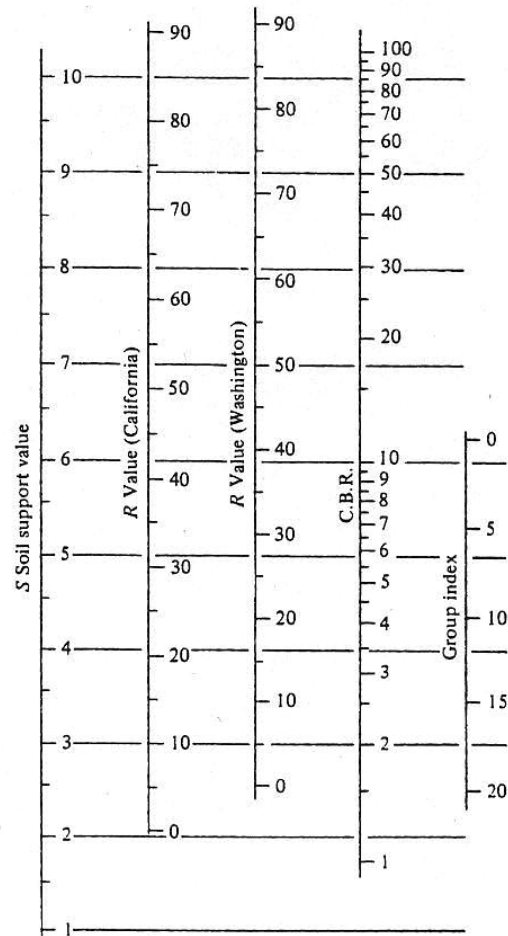


Figure 15.2. Generalized regional map of the United States. (From Van Til et al., NCHRP 128.)

Soil Support Value



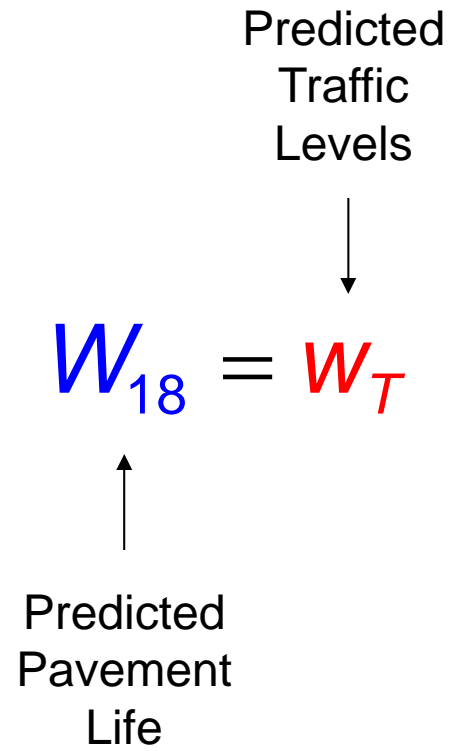
Flexible Performance

$$\log_{10} W_{18} = 9.36 \log_{10} (SN + 1) - 0.20 + \frac{\log_{10} \left(\frac{4.2 - p_t}{4.2 - 1.5} \right)}{0.4 + \frac{1094}{(SN + 1)^{5.19}}} + 2.32 \log_{10} (M_R) - 8.07$$

1986

↑
Seasonally
Adjusted
Resilient
Modulus

Design Equation



Design Equation

Predicted
Traffic
Levels



$$\log_{10} W_{18} = \log_{10} W_T$$



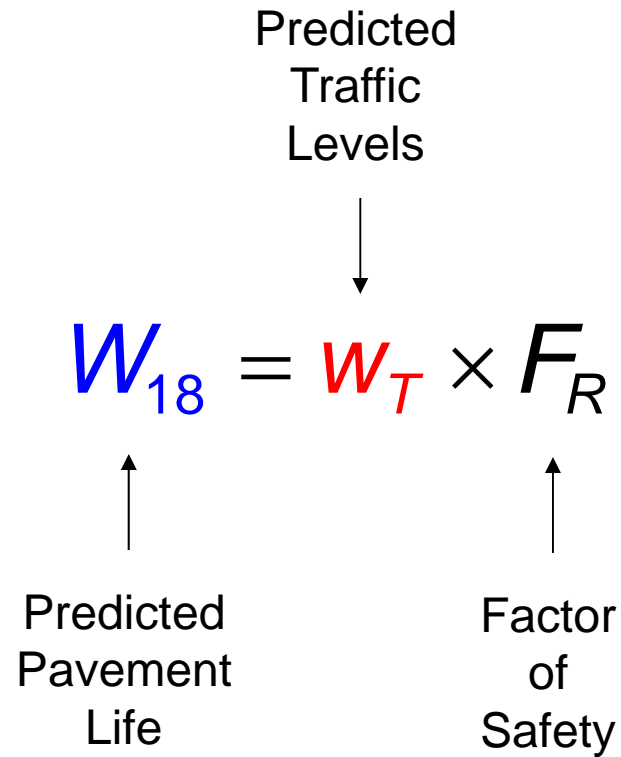
Predicted
Pavement
Life

Predicted Pavement Life

$$\log_{10} W_T = 9.36 \log_{10} (SN + 1) - 0.20 + \frac{\log_{10} \left(\frac{4.2 - p_t}{4.2 - 1.5} \right)}{0.4 + \frac{1094}{(SN + 1)^{5.19}}} + 2.32 \log_{10} (M_R) - 8.07$$

Reliability

Design Equation w/ FS



Design Equation w/ FS

Predicted
Traffic
Levels



$$\log_{10} W_t = \log_{10} W_T + \log_{10} F_R$$

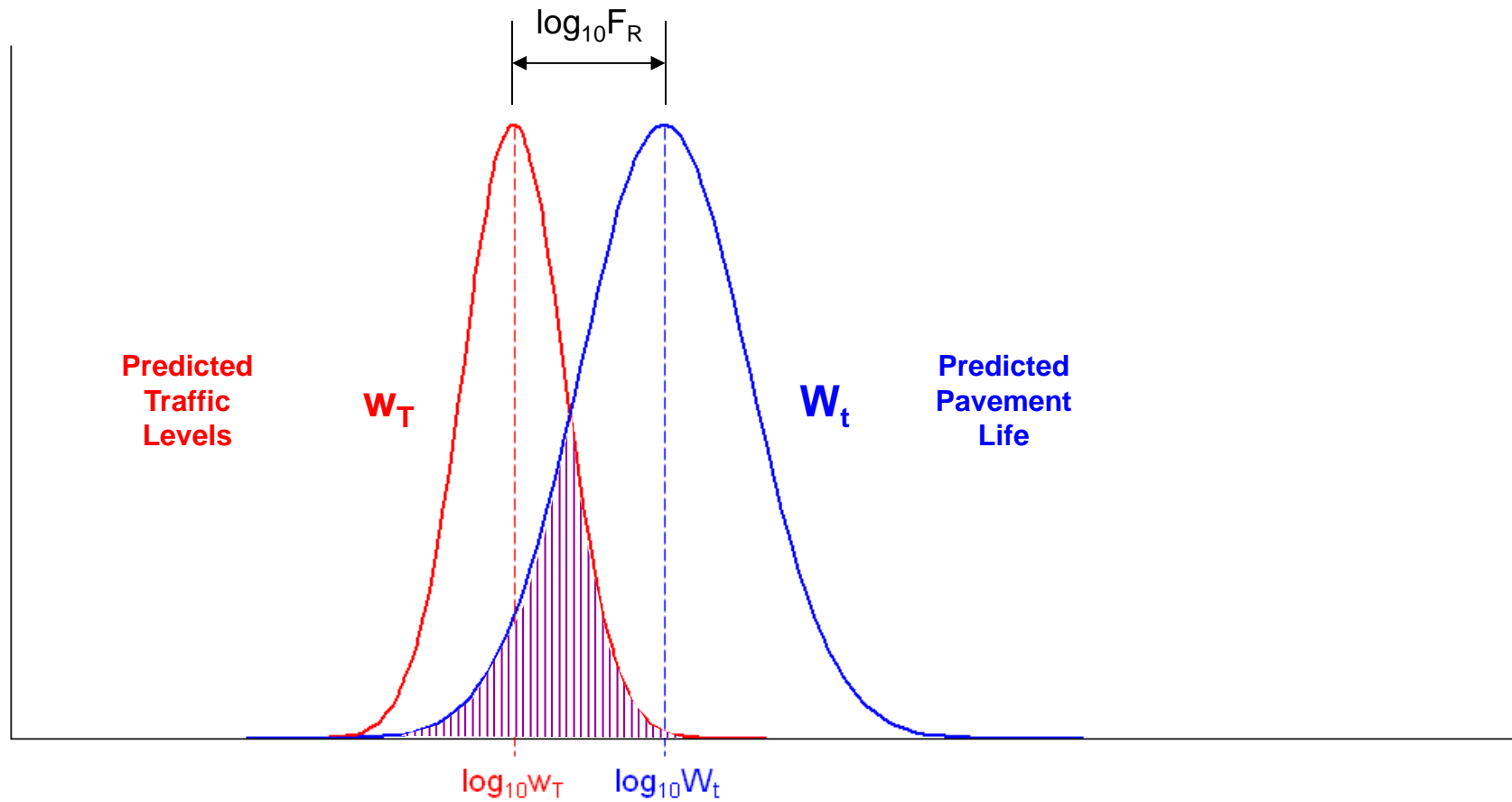
Predicted
Pavement
Life



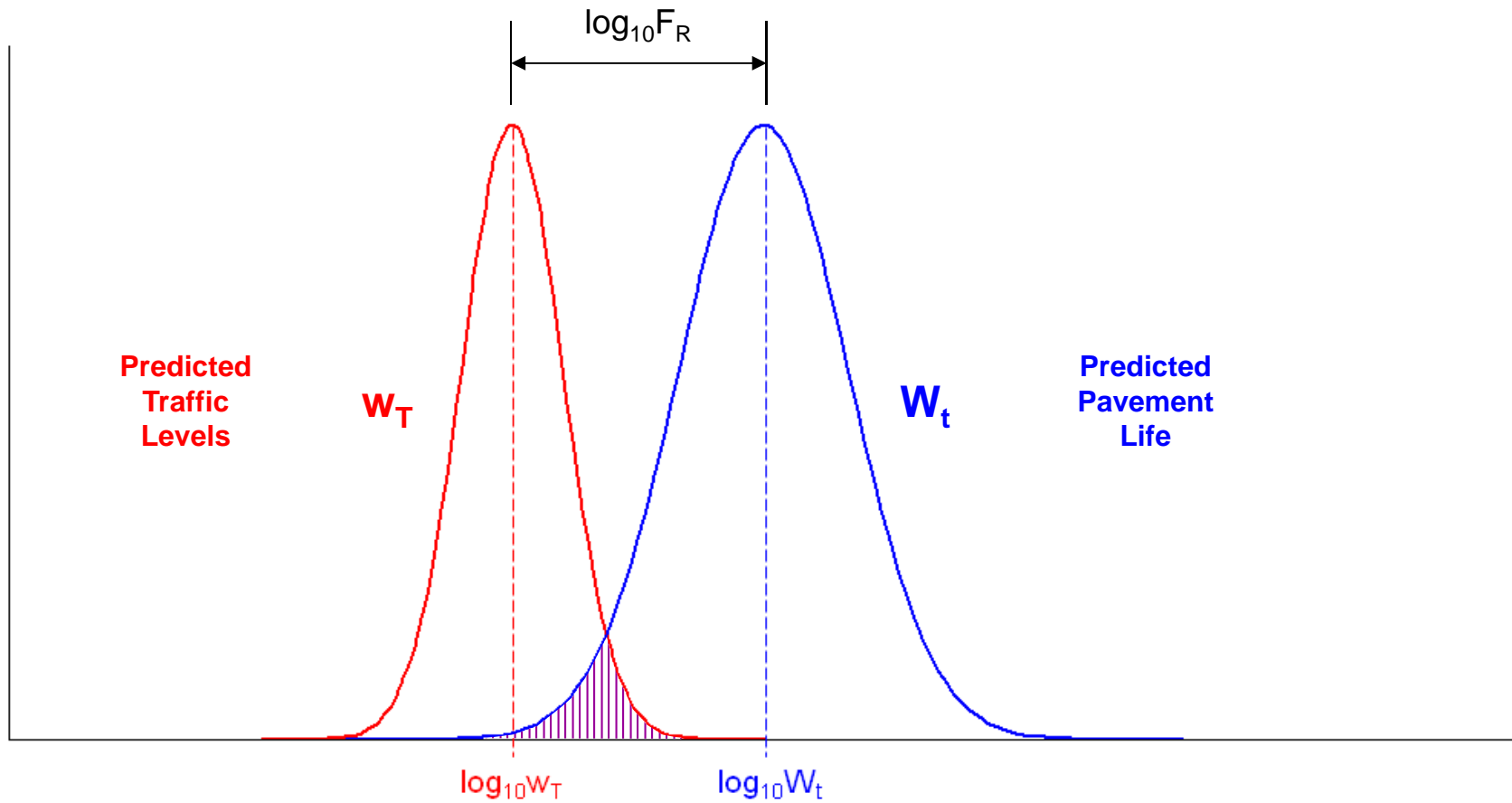
Factor
of
Safety



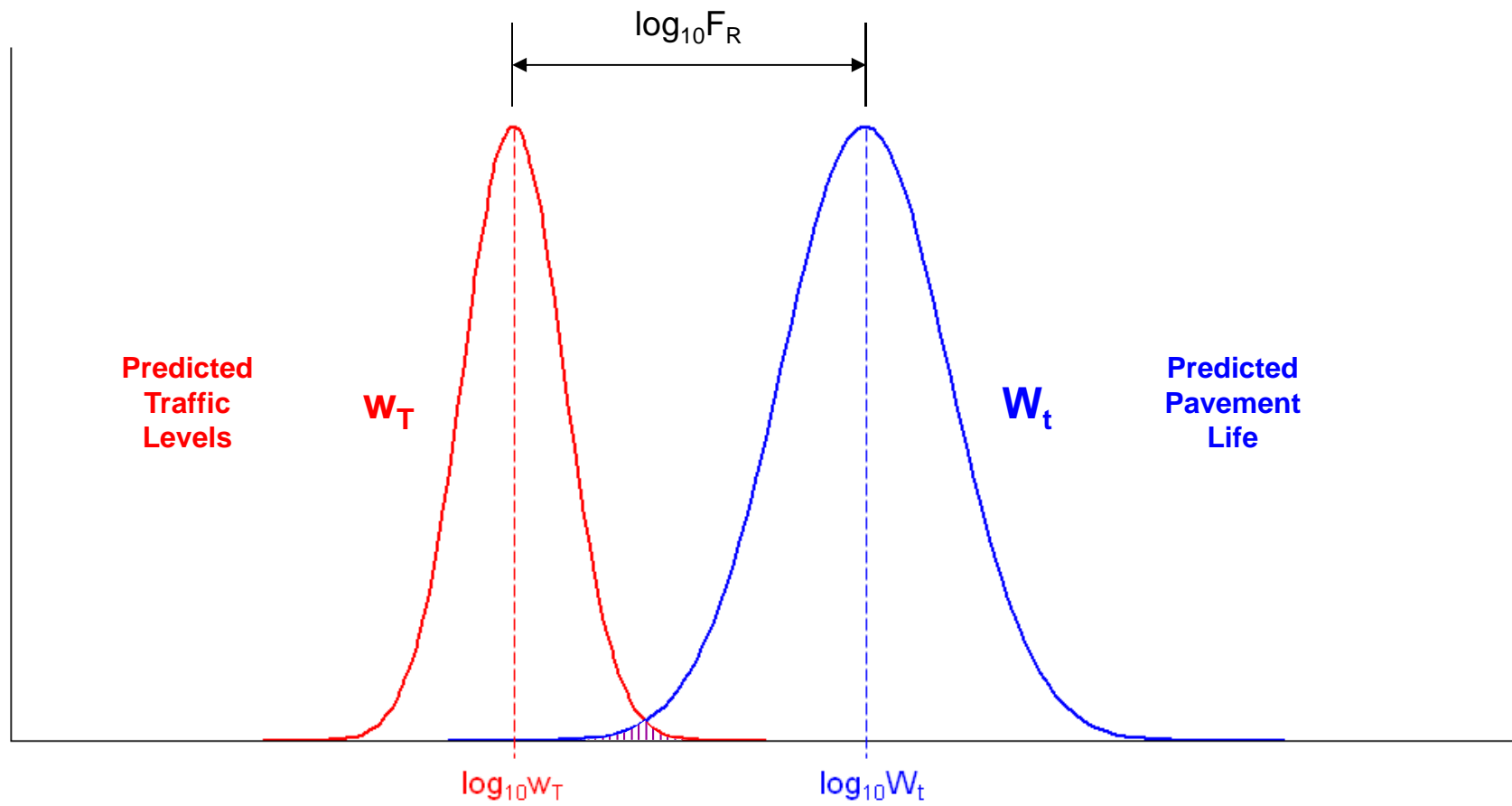
Reliability



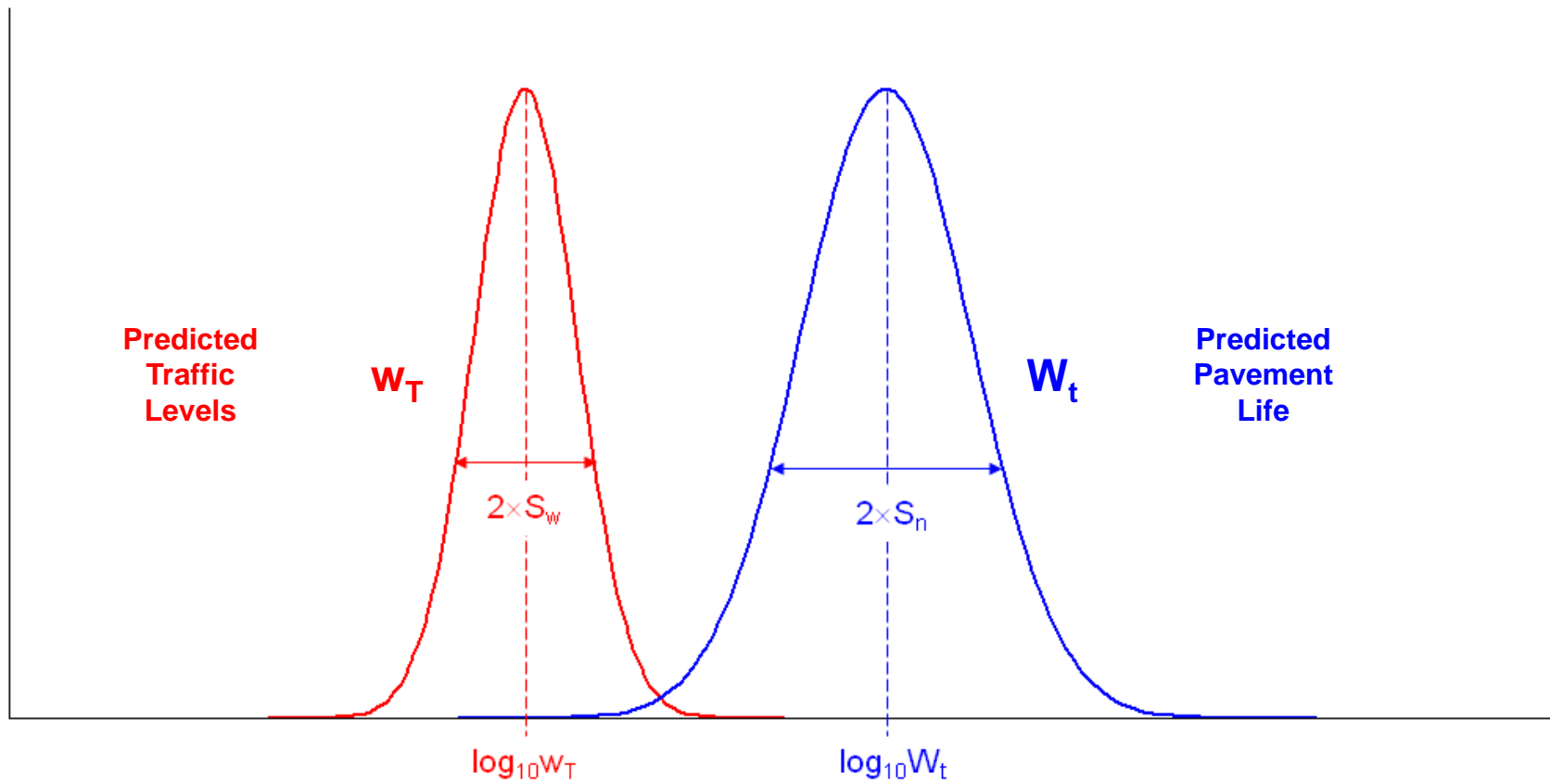
Reliability



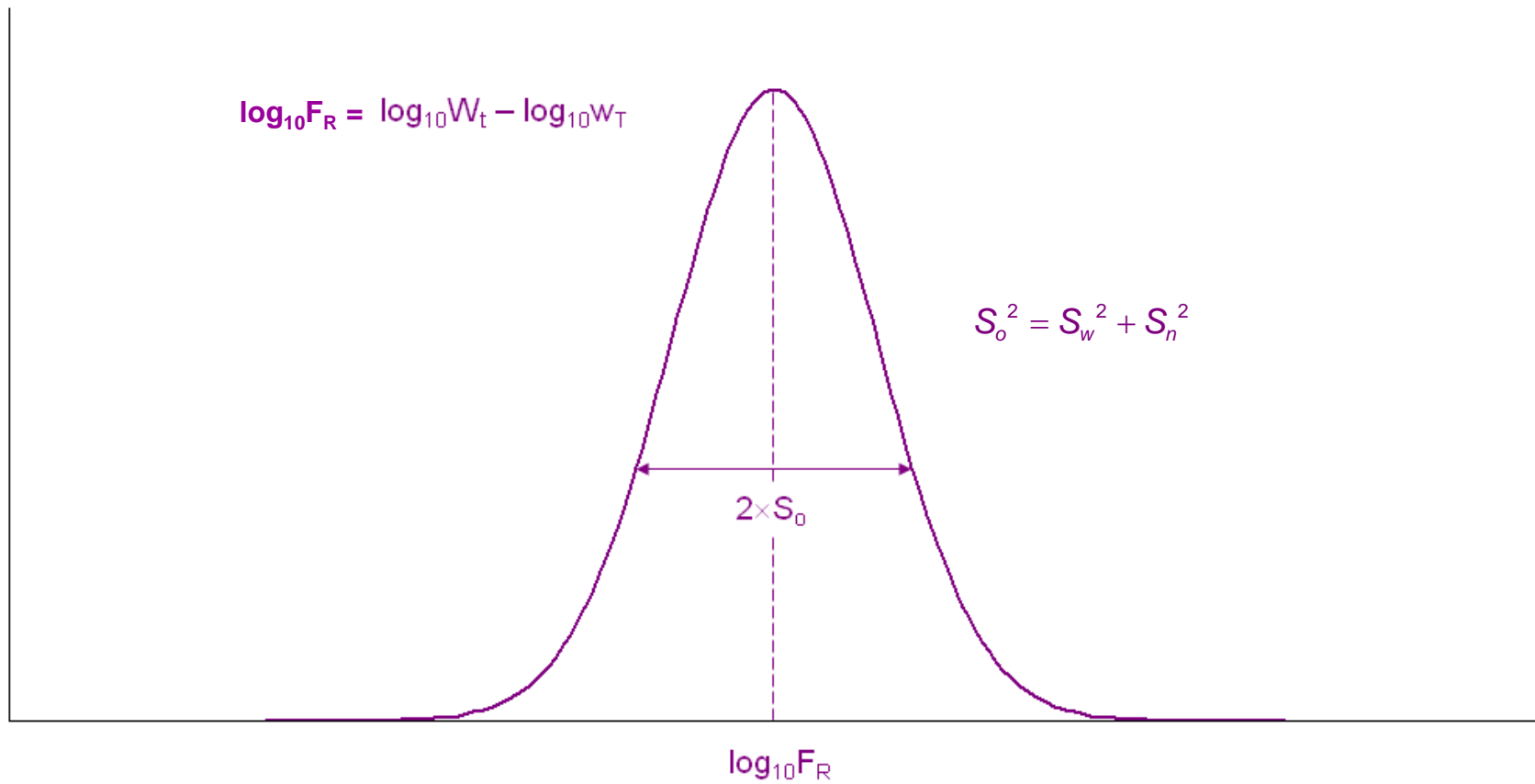
Reliability



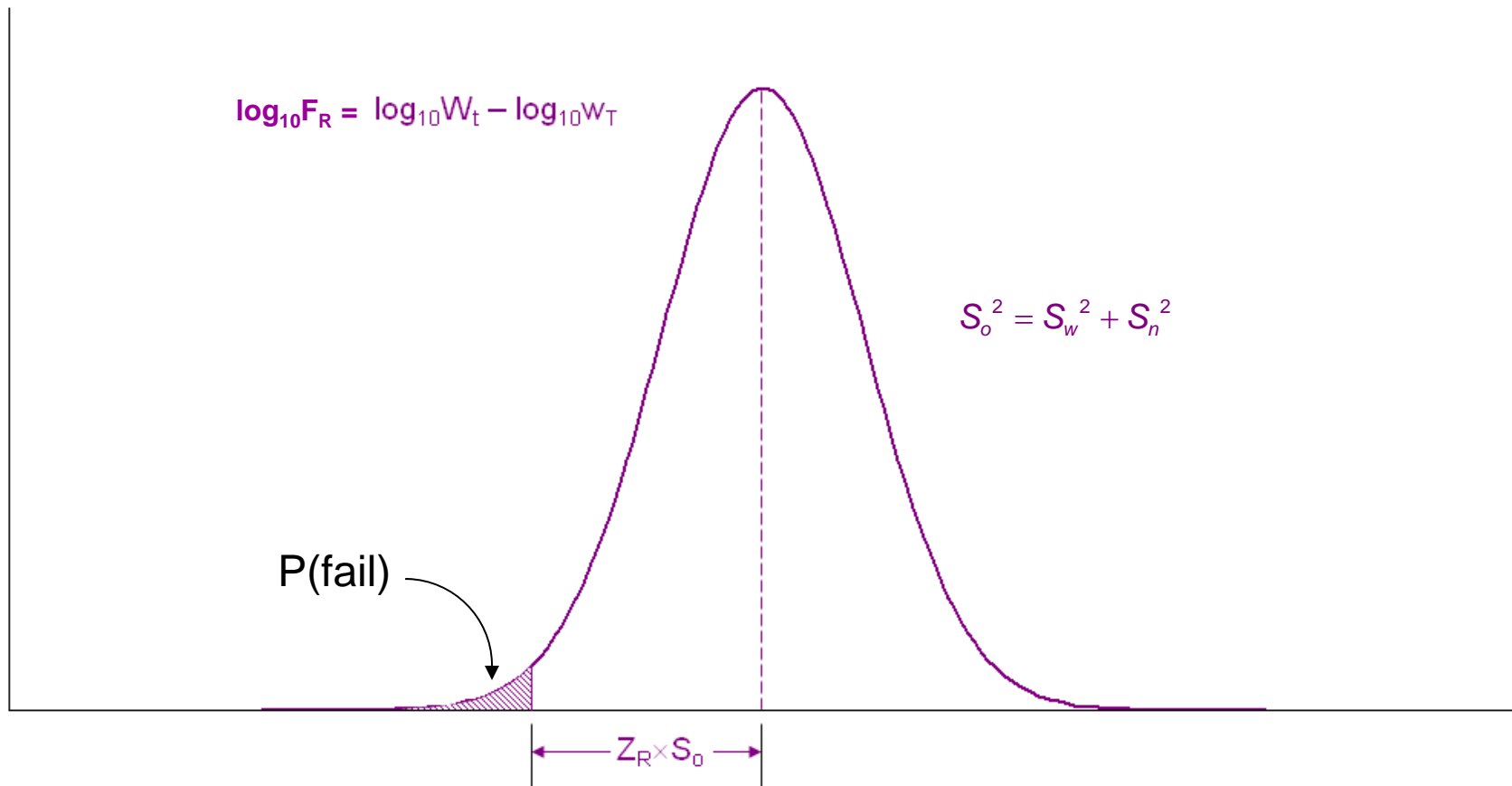
Reliability



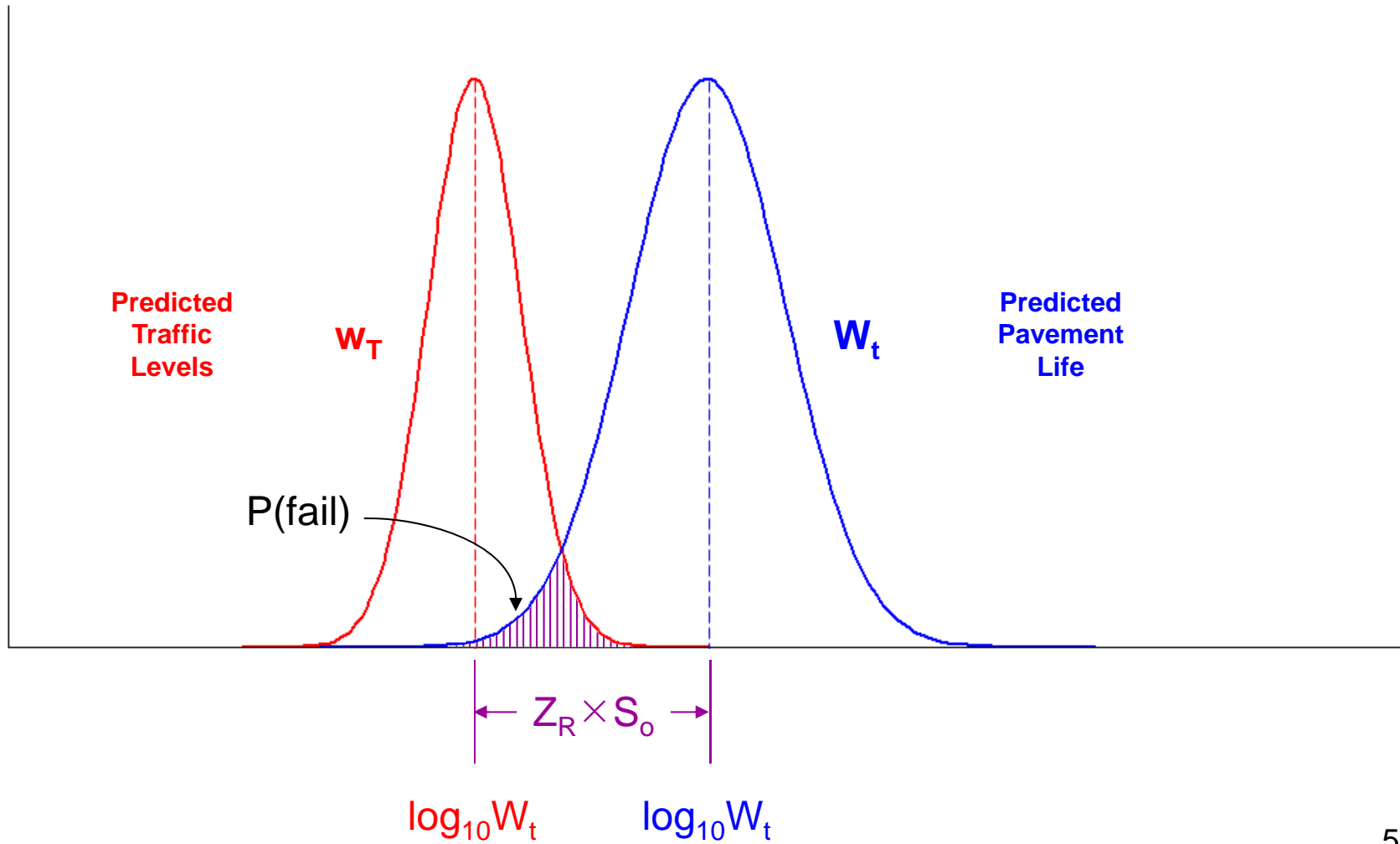
Reliability



Reliability



Reliability



Design Equation

Reliability



$$\log_{10} W_{18} = Z_R S_0 + 9.36 \log_{10} (SN + 1) - 0.20 + \frac{\log_{10} \left(\frac{4.2 - p}{4.2 - 1.5} \right)}{0.4 + \frac{1094}{(SN + 1)^{5.19}}} + 2.32 \log_{10} (M_R) - 8.07$$

1993

Recommended Reliability

Functional Classification	Urban	Rural
Interstate and Other Freeways	85 – 99.9%	80 – 99.9%
Principal Arterials	80 – 99%	75 – 99%
Collectors	80 – 95%	75 – 95%
Local	50 – 80%	50 – 80%

Reliability

Reliability (%)	Z_R
50	-0.000
60	-0.253
70	-0.524
75	-0.674
80	-0.841
85	-1.037
90	-1.282
91	-1.340
92	-1.405

Reliability (%)	Z_R
93	-1.476
94	-1.555
95	-1.645
96	-1.751
97	-1.881
98	-2.054
99	-2.327
99.9	-3.090
99.99	-3.750

Standard Deviations

Source	Flexible	Rigid
AASHO Road Test S_n	0.35	0.25
AASHO Road Test S_o	0.45	0.35
Typical Range for S_o	0.40 – 0.50	0.35 – 0.40