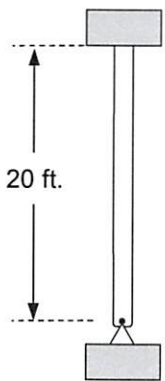


An HSS8 x 6 x 1/4 of A500 Grade C steel is 20 ft long, pinned at one end and fixed at the other. Compute the flexural buckling strength. Check for local stability.



* FROM TABLE 2-4: $F_y = 50 \text{ ksi}$

* FROM TABLE C-A-7.1: $K = 0.8$

* FROM TABLE 1-11 $\left[\begin{array}{l} A_g = 6.17 \text{ in}^2 \quad b/t = 22.8 \\ h/t = 31.3 \quad r_{\min} = 2.43 \text{ in} \end{array} \right.$

$$\frac{L_c}{r} = \frac{KL}{r} = \frac{0.8(20 \text{ ft}) (12 \text{ in/ft})}{2.43 \text{ in}} = 79.01 < 200 \text{ O.K.}$$

$$4.71 \sqrt{E/F_y} = 4.71 \sqrt{\frac{29,000 \text{ ksi}}{50 \text{ ksi}}} = 113.43 > \frac{L_c}{r} \quad \begin{array}{l} * \text{ USE} \\ \text{EQ E3-2} \end{array}$$

$$F_e = \frac{\pi^2 E}{(L_c/r)^2} = \frac{\pi^2 (29,000 \text{ ksi})}{(79.01)^2} = 45.85 \text{ ksi}$$

$$F_n = F_y \left(0.658^{F_y/F_e} \right) = 50 \text{ ksi} \left(0.658^{50/45.85} \right) = 31.68 \text{ ksi}$$

$$\phi P_n = \phi F_n A_g = 0.9 (31.68 \text{ ksi}) (6.17 \text{ in}^2) = \underline{175.9 \text{ k}}$$

CHECK FOR LOCAL STABILITY

FROM TABLE B.4.1a $\lambda_r = 1.40 \sqrt{\frac{E}{F_y}} = 1.40 \sqrt{\frac{29,000 \text{ ksi}}{50 \text{ ksi}}} = 33.72$

$$\lambda = h/t = 31.3 < \lambda_r \quad \text{NONSLENDER} \quad \underline{\text{O.K.}}$$

$$\underline{\underline{\phi P_n = 175.9 \text{ k}}}$$