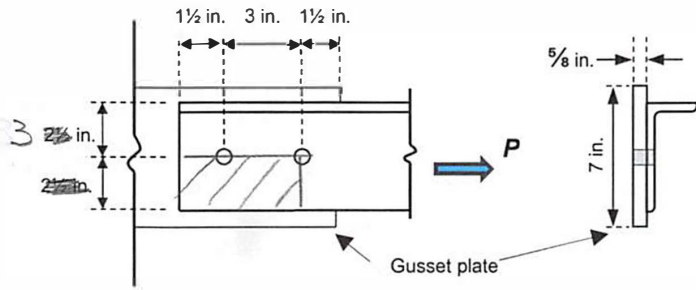


Compute the design strength of an  $L5 \times 3 \times \frac{1}{4}$  tension member connected to a gusset plate by two  $\frac{7}{8}$ -in.-diameter bolts in the standard holes. **A572 Grade 50** steel ( $F_y = 50 \text{ ksi}$ ,  $F_u = 65 \text{ ksi}$ ) is used. Assume  $U = 0.60$



FROM TABLE 1-5  
 $A = 1.94 \text{ in}^2$   
 $d_{\text{hole}} = \frac{7}{8} \text{ in} + \frac{1}{8} \text{ in} = 1 \text{ in}$

YIELD - GROSS AREA

$$\phi P_n = \phi F_y A_g = 0.9(50 \text{ ksi}) 1.94 \text{ in}^2 = \underline{87.3 \text{ K}}$$

RUPTURE - NET AREA

$$\phi P_n = \phi F_u A_n U$$

$$A_n = A_g - 1 \text{ HOLE} = 1.94 \text{ in}^2 - (1 \text{ in}) \frac{1}{4} \text{ in} = 1.69 \text{ in}^2$$

$$= 0.75(65 \text{ ksi}) 0.6(1.69 \text{ in}^2)$$

$$= \underline{49.43 \text{ K}}$$

BLOCK SHEAR

$$A_{gv} = (3 \text{ in} + 1 \frac{1}{2} \text{ in}) \frac{1}{4} \text{ in} = 1.125 \text{ in}^2$$

$$A_{nv} = A_{gv} - 1 \frac{1}{2} \text{ HOLES} = 1.125 \text{ in}^2 - 1 \frac{1}{2} (1 \text{ in}) \frac{1}{4} \text{ in} = 0.75 \text{ in}^2$$

$$A_{nt} = 2 \text{ in} (\frac{1}{4} \text{ in}) - \frac{1}{2} (1 \text{ in}) \frac{1}{4} \text{ in} = 0.375 \text{ in}^2$$

$$\phi P_n = \text{MIN} [0.6 F_u A_{nv}, 0.6 F_y A_{gv}] + F_u A_{nt}$$

$$= \text{MIN} [0.6(65 \text{ ksi}) 0.75 \text{ in}^2, 0.6(50 \text{ ksi}) 1.125 \text{ in}^2] + 65 \text{ ksi} (0.375 \text{ in}^2)$$

$$= \text{MIN} [29.25 \text{ K}, 33.75 \text{ K}] + 24.38 \text{ K} = 53.63 \text{ K}$$

$$\phi P_n = 0.75(53.63 \text{ K}) = \underline{40.22 \text{ K}} \quad * \text{ BLOCK SHEAR CONTROLS}$$