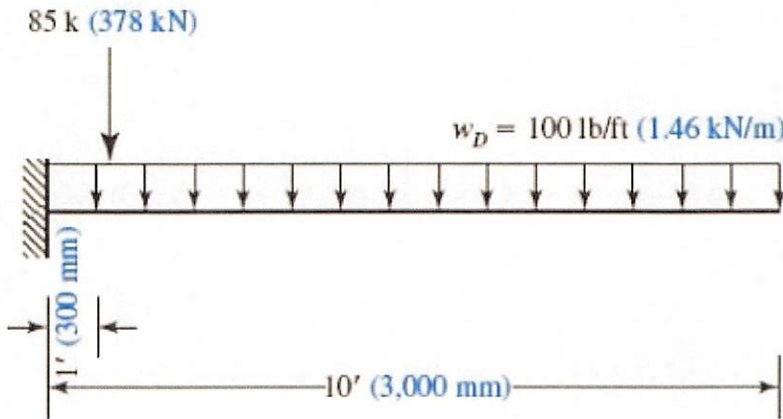
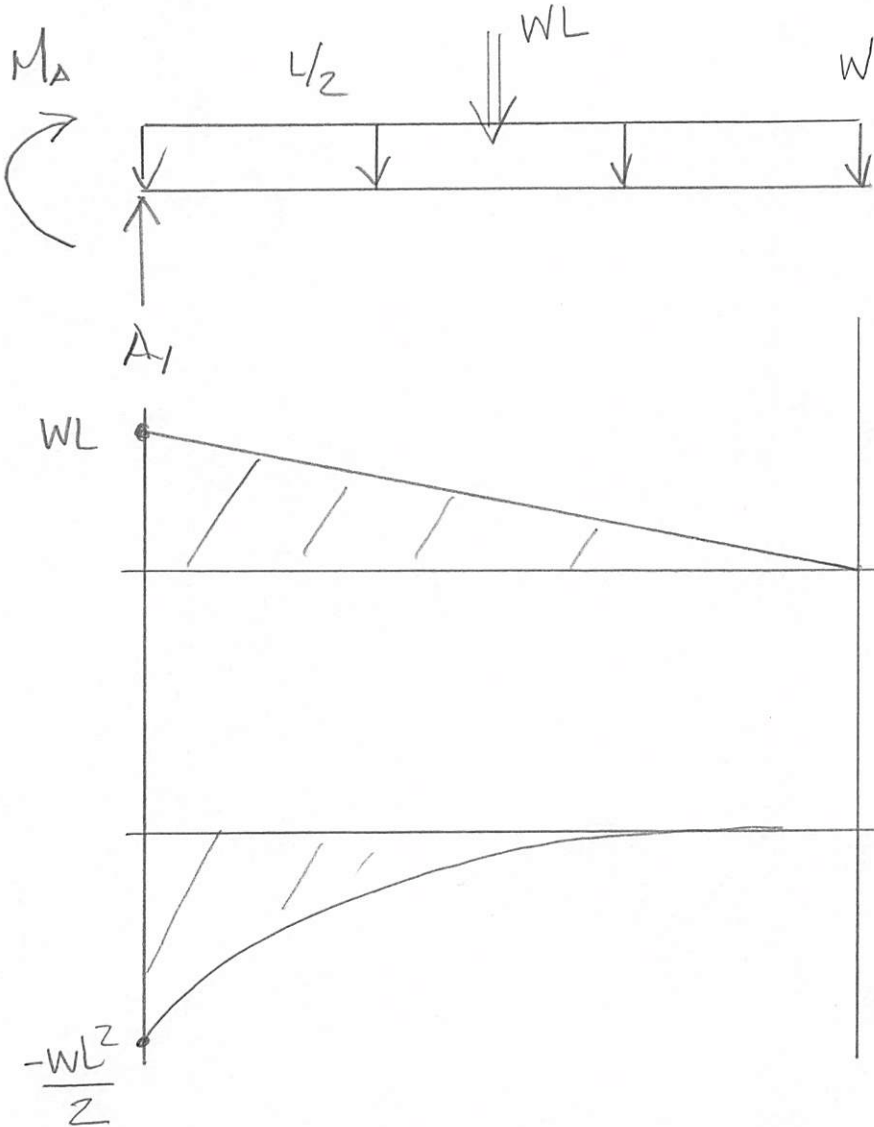


1/4

5.8-4 The cantilever beam shown in [Figure P5.8-4](#) is a $W16 \times 45$ ($W410 \times 67$) of A992 steel ($F_y = 50 \text{ ksi}$ (345 MPa)). There is no lateral support other than at the fixed end. Use an unbraced length equal to the span length and determine whether the beam is adequate. The uniform load is a service dead load that includes the beam weight, and the concentrated load is a service live load.



CANTILEVER $C_b = 1.0$
 AISCF-1 (16.1-52)
 NOTE (c)



$$\sum M_A = 0 = +WL\left(\frac{L}{2}\right) + M$$

$$M = -\frac{WL^2}{2}$$

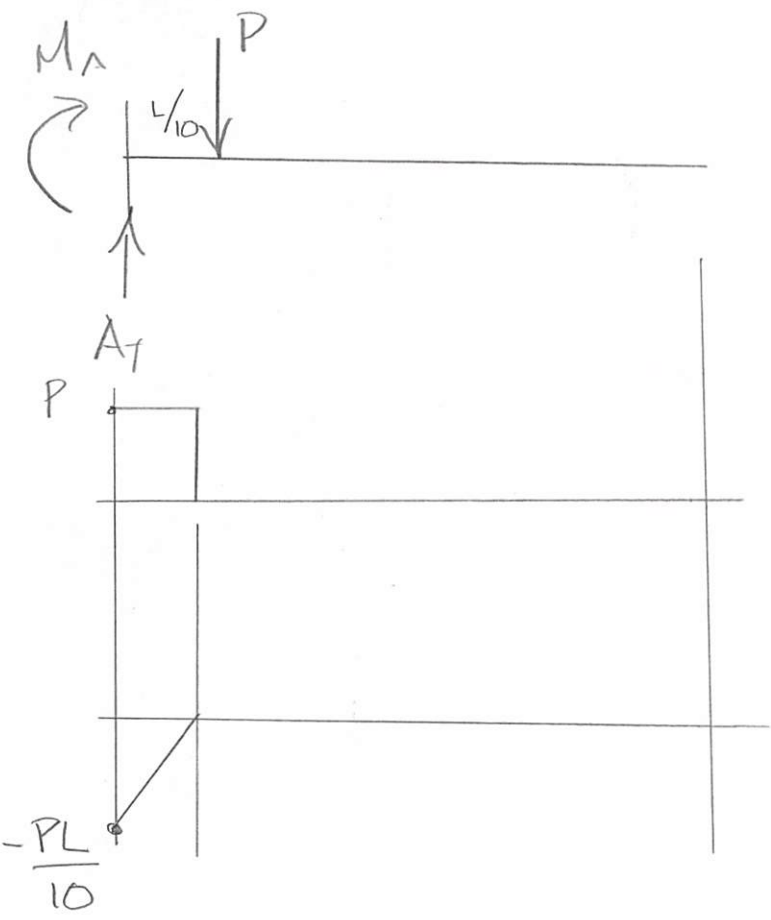
$$\sum F_y = 0 = A_1 - WL$$

$$A_1 = WL$$

5.8-4

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$$M_A = -\frac{PL}{10} \quad A_T = P$$



W16 X 45
 * FROM TABLE 1-1 (1-22)
 $Z_x = 82.3 \text{ IN}^3$ $S_x = 72.7 \text{ IN}^3$
 $h/t_w = 41.1$ $d = 16.1$
 $t_w = 0.345 \text{ IN}$

FROM "Z TABLE" TABLE 3-2 (3-25)

$$M_p / \Omega_b = 205 \text{ kft} \quad \phi_b M_p = 309 \text{ kft}$$

$$L_p = 5.55 \text{ ft} \quad L_r = 16.5 \text{ ft} \quad L_b = 10 \text{ ft} > L_p$$

$$* L_p < L_b < L_r$$

$$M_n = C_b \left[M_p - (M_p - 0.75 S_x F_y) \left(\frac{L_b - L_p}{L_r - L_p} \right) \right] \leq M_p$$

$$M_p = F_y Z_x = 50 \text{ ksi} (82.3 \text{ IN}^3) = 4,115 \text{ k}\cdot\text{in} = 342.92 \text{ kft}$$

$$0.7 F_y S_x = 0.7 (50 \text{ ksi}) (72.7 \text{ IN}^3) = 2,544.5 \text{ k}\cdot\text{in}$$

5.8-4

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$$M_n = \left[4115 - (4115 - 2544.5) \left[\frac{10 - 5.55}{165 - 5.55} \right] \right] = 3,476.76 \text{ k}\cdot\text{in}$$
$$= \underline{289.73 \text{ kft}}$$

LRFD $W = 1.2W_D = 1.2(0.1 \text{ k/ft}) = 0.12 \text{ k/ft}$

$$P = 1.6P_L = 1.6(85 \text{ k}) = 136 \text{ k}$$

$$M_U = M_{\text{MAX}} = \frac{WL^2}{2} + \frac{PL}{10} = \frac{(0.12 \text{ k/ft})(10 \text{ ft})^2}{2} + \frac{136 \text{ k}(10 \text{ ft})}{10}$$
$$= \underline{142 \text{ kft}}$$

$$\phi_b M_n = 0.9(289.73 \text{ kft}) = 260.75 \text{ kft} > M_U \quad \underline{\underline{\text{o.k.}}}$$

ASD $W = W_D$ $P = P_L$

$$M_U = M_{\text{MAX}} = \frac{(0.1 \text{ k/ft})(10 \text{ ft})^2}{2} + \frac{85 \text{ k}(10 \text{ ft})}{10} = 90 \text{ k}$$

$$\frac{M_n}{S_b} = \frac{289.73 \text{ kft}}{1.67} = 173.84 \text{ kft} > 90 \text{ k} \quad \underline{\underline{\text{o.k.}}}$$

CHECK SHEAR

$$\frac{h}{t_w} = 41.1 < 2.24 \sqrt{\frac{E}{F_y}} = 53.95$$

$$C_v = 1.0$$

$$\phi_v = 1.0$$

$$\Omega_v = 1.5$$

5.8-4

4/4

$$V_n = 0.6 F_y A_w C_{v1} = 0.6 (50 \text{ ksi}) (16.1 \text{ in}) (0.345 \text{ in}) (1.0) \\ = 166.6 \text{ k}$$

LRFD

$$\phi_v V_n = V_n = 166.6 \text{ k}$$

$$V_u = WL + P = (1.2 W_D \text{ k/ft}) (10 \text{ ft}) + 1.6 P_L \\ = (0.12 \text{ k/ft}) (10 \text{ ft}) + 1.6 (85 \text{ k}) = 137.2 \text{ k}$$

$$\underline{\phi_v V_n > V_u} \quad \underline{\underline{0. \text{ k.}}}$$

ASD

$$\frac{V_n}{\Omega_v} = \frac{166.6 \text{ k}}{1.5} = 111.1 \text{ k}$$

$$V_u = 0.1 \text{ k/ft} (10 \text{ ft}) + 85 \text{ k} = 86 \text{ k}$$

$$V_u < \frac{V_n}{\Omega_v} \quad \underline{\underline{0. \text{ k.}}}$$