

5.6-1 A W12 × 65 (W310 × 97) is used as a simply supported, uniformly loaded beam with a span length of 50 feet (15,250 mm) and continuous lateral support. The yield stress,  $F_y$ , is 50 ksi (345 MPa). If the ratio of live load to dead load is 3, compute the available strength and determine the maximum total service load, in kips/ft (kN/m), that can be supported.

FROM TABLE 1-1 (1-26)

$$\left[ \begin{array}{l} d = 12.1 \text{ in} \quad t_f = 0.605 \text{ in} \\ \frac{b_f}{2t_f} = 9.92 \quad \frac{h}{t_w} = 24.9 \quad S_x = 87.9 \text{ in}^3 \\ Z_x = 96.8 \text{ in}^3 \quad I_y = 174 \text{ in}^4 \quad r_y = 3.02 \text{ in} \\ r_{ts} = 3.38 \text{ in} \quad h_o = 11.5 \text{ in} \quad J = 2.18 \text{ in}^4 \\ C_w = 5780 \text{ in}^6 \end{array} \right.$$

CHECK FOR COMPACTNESS

FLANGE  $\frac{b_f}{2t_f} = 9.92 > 0.38 \sqrt{E/F_y} = 9.15$  NONCOMPACT

WEB  $\frac{h}{t_w} = 24.9 < 3.76 \sqrt{E/F_y} = 90.55$  COMPACT ✓

\* COMPUTE STRENGTH BASED ON FLANGE BUCKLING

$$\lambda_p = 0.38 \sqrt{E/F_y} = 9.15 \quad \lambda_r = 1.0 \sqrt{E/F_y} = 24.08$$

$$\therefore \lambda_p < \lambda < \lambda_r$$

$$M_p = F_y Z_x = (50 \text{ ksi})(96.8 \text{ in}^3) = 4,840 \text{ k} \cdot \text{in}$$

$$M_n = M_p - (M_p - 0.7F_y S_x) \left[ \frac{\lambda - \lambda_{pf}}{\lambda_{ryf} \lambda_{pff}} \right]$$

$$= 4,840 \text{ k-in} - (4,840 \text{ k-in} - 0.7(50 \text{ ksi})(87.9 \text{ in}^3)) \left[ \frac{9.92 - 9.15}{24.08 - 9.15} \right]$$

$$= 4,749.04 \text{ k-in} = \underline{395.75 \text{ k-ft}}$$

$$\phi_b M_n = 0.9(395.75 \text{ k-ft}) = 356.18 \text{ k-ft}$$

LRFD LOADS     $L = 3D$      $W_U = 1.2W_D + 1.6(3W_D) = 6W_D$

$$M_{\text{MAX}} = \frac{WL^2}{8} = \frac{6W_D(50 \text{ ft})^2}{8} = 1,875 W_D \text{ k-ft}$$

$$\therefore 356.18 \text{ k-ft} = 1,875 W_D \text{ k-ft} \quad \underline{W_D = 0.19 \text{ k/ft}}$$

$$\underline{W_L = 0.57 \text{ k/ft}}$$

$$W = W_D + W_L = \underline{0.76 \text{ k/ft}}$$

ASD

$$M_a = \frac{M_n}{\Omega_b} = \frac{395.75 \text{ k-ft}}{1.67} = \underline{236.98 \text{ k-ft}}$$

$$W = W_D + W_L = W_D + 3W_D = 4W_D$$

$$M_{\text{MAX}} = \frac{WL^2}{8} = \frac{4W_D(50 \text{ ft})^2}{8} = 1,250 W_D \text{ k-ft}$$

$$1,250 W_D \text{ k-ft} = 236.98 \text{ k-ft} \quad \therefore W_D = 0.19 \text{ k/ft}$$

$$\Rightarrow \underline{W = 0.758 \text{ k/ft}}$$