

5.5-6 A W12 x 30 (W310 x 44.5) of A992 steel ($F_y = 50 \text{ ksi} (345 \text{ MPa})$) has an unbraced length of 10 feet (3,000 mm). Using $C_b = 1.0$,

- a. Compute L_p and L_r . Use the equations in Chapter F of the AISC Specification. Do not use any of the design aids in the Manual.
- b. Compute the flexural design strength, $\phi_b M_n$.
- c. Compute the allowable flexural strength M_n / Ω_b .

FROM TABLE 1-1 (1-26)

$A = 8.79 \text{ in}^2$	$d = 12.3 \text{ in}$
$t_f = 0.440 \text{ in}$	$S_x = 38.6 \text{ in}^3$
$I_y = 20.3 \text{ in}^4$	$J = 0.457 \text{ in}^4$
$C_w = 720 \text{ in}^6$	$r_y = 1.52 \text{ in}$
$Z_x = 43.1 \text{ in}^3$	

$$L_p = 1.76 r_y \sqrt{E/F_y} = 1.76 (1.52 \text{ in}) \sqrt{\frac{29,000 \text{ ksi}}{50 \text{ ksi}}} = 64.43 \text{ in}$$

$$L_b = 10 \text{ ft} (12 \text{ in/ft}) = 120 \text{ in} > L_p$$

COMPUTE L_r

$$\frac{r_{ts}^2}{S_x} = \frac{\sqrt{I_y C_w}}{S_x} = \frac{\sqrt{20.3 \text{ in}^4 (720 \text{ in}^6)}}{38.6 \text{ in}^3} = 3.132 \text{ in}^2$$

$C = 1.0$ FOR DOUBLY SYMMETRIC I SHAPES

$$h_o = d - t_f = 12.3 \text{ in} - 0.44 \text{ in} = 11.86 \text{ in}$$

$$\frac{J_c}{S_x h_o} = \frac{0.457 \text{ in}^4 (1.0)}{38.6 \text{ in}^3 (11.86 \text{ in})} = 0.0010$$

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2/2

$$L_r = 1.95 r_{ts} \frac{E}{0.7F_y} \sqrt{\frac{J_c}{S_x h_o} + \sqrt{\left(\frac{J_o}{S_x h_o}\right)^2 + 6.76 \left(\frac{0.7F_y}{E}\right)^2}}$$

$$= 1.95 (1.7697 \text{ m}) \frac{29,000}{0.7(50)} \sqrt{0.0010 + \sqrt{(0.0010)^2 + 6.76 \left(\frac{0.7(50)}{29,000}\right)^2}}$$

$$= 187.31 \text{ IN}$$

$L_b < L_r \Rightarrow$ USE AISC EQN. F2-2

$$M_n = C_b \left[M_p - (M_p - 0.75 F_y S_x) \left(\frac{L_b - L_p}{L_r - L_p} \right) \right] \leq M_p$$

CHECK FOR COMPACT

$$\frac{b_f}{2t_f} = 7.41 \quad 0.38 \sqrt{\frac{E}{F_y}} = 9.15 \quad \lambda < \lambda_p \text{ COMPACT}$$

$$\frac{h}{t_w} = 41.8 \quad 3.76 \sqrt{\frac{E}{F_y}} = 90.55 \quad \lambda < \lambda_p \text{ COMPACT}$$

$$M_p = F_y Z_x = 50 \text{ ksi} (43.1 \text{ in}^3) = 2,155 \text{ k}\cdot\text{IN}$$

$$M_n = 1.0 \left[2,155 \text{ k}\cdot\text{IN} - (2,155 \text{ k}\cdot\text{IN} - 0.7 (50 \text{ ksi}) (38.6 \text{ in}^3) \right] \left[\frac{120 - 64.43}{187.31 - 64.43} \right]$$

$$= 1,791.12 \text{ k}\cdot\text{IN}$$

$$\phi_b M_n = 1,612.0 \text{ k}\cdot\text{IN} = \underline{\underline{134.33 \text{ kft}}}$$