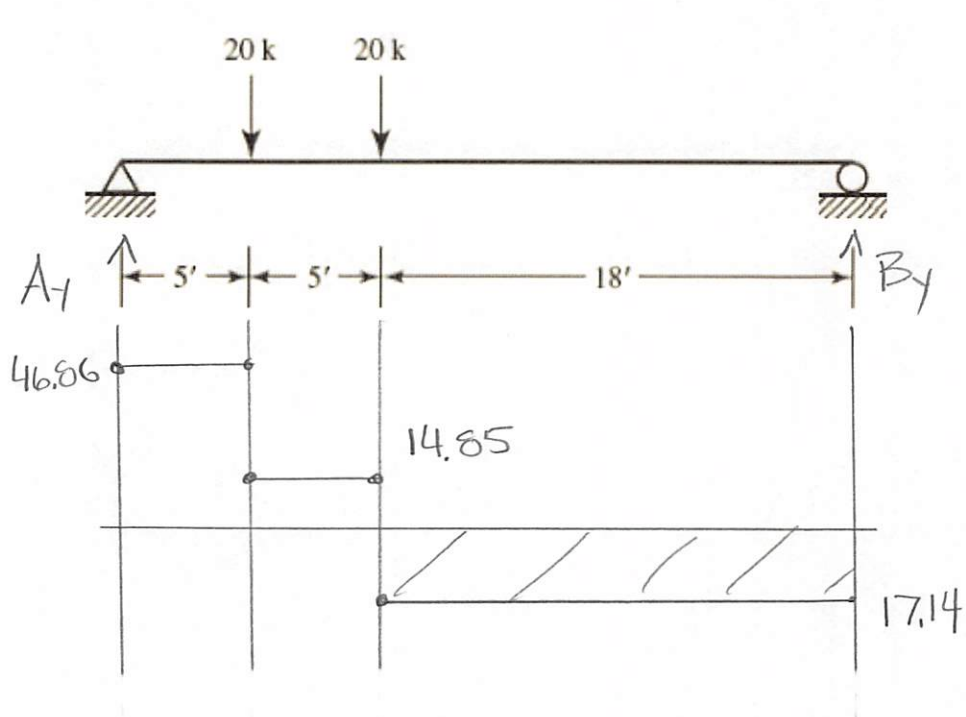


5.10-6 The beam shown in [Figure P5.10-6](#) has lateral support at the ends only. The concentrated loads are live loads. Use A992 steel ($F_y = 50$ ksi) and select a shape. Do not check deflections. Use $C_b = 1.0$ (this is conservative).



$L_b = 28$ ft

$P_u = 1.6(20k) = 32k$

$\sum M_B = 0 = P_u(18' + 23') - A_y(28')$

$A_y = 46.86k$

$B_y = 17.14k$

$M_{MAX} = 17.14k(18') = 308.6$ kft

FROM TABLE 3-10 (3-119) WITH $L_b = 28$ ft & $M_u = 309$ kft

TRY W14 x 74 $\phi_b M_n = 318$ kft $> M_u$ O.k.

* CHECK BEAM WT $M_u = 308.6$ kft + $\frac{1.2(0.074k/ft)(28ft)^2}{8} = 317.3$ kft O.k.

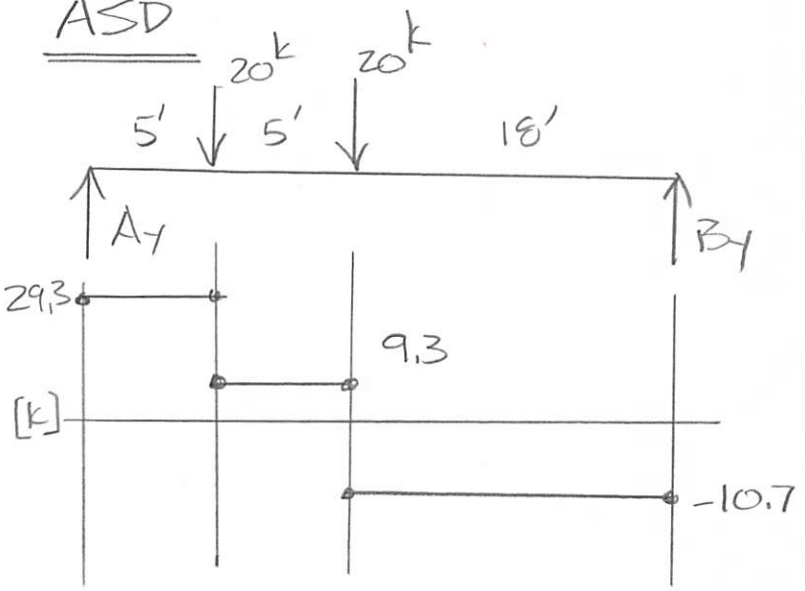
* CHECK SHEAR FROM Z_x TABLE

$\phi_v V_n = 192$ k > 46.86 O.k.

USE W14 x 74

ASD

$L_b = 28 \text{ ft}$



$$\begin{aligned} \sum M_B = 0 &= 20k(18' + 23') \\ &+ -A_y(28') \\ A_y &= 29.3k \\ B_y &= 10.7k \end{aligned}$$

$M_{MAX} = 10.7k(18 \text{ ft}) = 192.6 \text{ kft}$

FROM TABLE 3-10 (3-21) WITH $L_b = 28'$ & M_U

W14 x 74

$\frac{M_n}{\Omega_b} = \underline{212 \text{ kft}} > M_U \quad \underline{\text{O.K.}}$

* CHECK BEAM WT

$M_U = 192.6 + \frac{(0.074 \text{ k/ft})(28 \text{ ft})^2}{8}$
 $= \underline{199.9 \text{ kft}} \quad \underline{\text{O.K.}}$

* CHECK SHEAR FROM Z_x TABLE (3-25)

$\frac{V_n}{\Omega_v} = 128k > 29.3k \quad \underline{\text{O.K.}}$

USE W14 x 74