

4.4-2 A W21 × 101 (W530 × 150) is used as compression member with one end fixed and the other end free. The length is 10 feet (3,000 mm). What is the nominal compressive strength if $F_y = 50$ ksi (345 MPa)? Note that this is a slender-element compression member, and the equations of AISC Section E7 must be used.

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$K = 2.1$ FROM TABLE C-A-7.1 (16.1-650)

W21 × 101 ^(c) $\left[\begin{array}{l} A_g = 29.8 \text{ in}^2 \quad r_x = 9.02 \text{ in} \quad r_y = 2.89 \text{ in} \\ t_w = 0.50 \text{ in} \quad b_f = 12.3 \text{ in} \quad t_f = 0.80 \text{ in} \\ k_{DES} = 1.3 \text{ in} \quad d = 21.4 \text{ in} \quad k_{DES} = 1.3 \text{ in} \end{array} \right.$

$$\lambda_r = \frac{L_c}{r} = \frac{2.1(10 \text{ ft})(12 \text{ in/ft})}{2.89 \text{ in}} = 87.20$$

$$4.71 \sqrt{E/F_y} = 4.71 \sqrt{\frac{29,000 \text{ ksi}}{50 \text{ ksi}}} = 113.43$$

$$\frac{L_c}{r} < 4.71 \sqrt{E/F_y} \Rightarrow \text{USE EQ. E3-2}$$

$$F_e = \frac{\pi^2 E}{(\lambda_r)^2} = \frac{\pi^2 (29,000 \text{ ksi})}{(87.20)^2} = 37.64 \text{ ksi}$$

$$F_n = F_y \left(0.658^{F_y/F_e} \right) = 50 \text{ ksi} \left(0.658^{50/37.64} \right) = 28.68$$

$$P_n = F_n A_g = 28.68 \text{ ksi} (29.8 \text{ in}^2) = \underline{854.52 \text{ k}}$$

4.4-2] CHECK WIDTH-THICKNESS RATIOS TABLE B4.1a (16,1-21) 2/2

$$\frac{b_f}{2t_f} = \frac{12.3 \text{ IN}}{2(0.80 \text{ IN})} = 7.69$$

$$\lambda_r = 0.56 \sqrt{\frac{E}{F_y}}$$

$$= 0.56 \sqrt{\frac{29,000 \text{ ksi}}{50 \text{ ksi}}} = 13.49$$

FLANGE
 $\frac{b_f}{t} < \lambda_r$ NOT SLENDER

$$\frac{h}{t_w} = \frac{d - 2k_{des}}{t_w}$$

$$\lambda_r = 1.49 \sqrt{\frac{E}{F_y}}$$

$$= \frac{21.4 \text{ IN} - 2(1.30 \text{ IN})}{0.50 \text{ IN}}$$

$$= 1.49 \sqrt{\frac{29,000 \text{ ksi}}{50 \text{ ksi}}} = 35.88$$

$= 37.60 > \lambda_r$ WEB IS SLENDER*

$$\lambda_r \sqrt{\frac{F_y}{F_n}} = 35.88 \sqrt{\frac{50 \text{ ksi}}{28.68 \text{ ksi}}} = 47.37$$

$\lambda = 37.6 < \lambda_r \sqrt{\frac{F_y}{F_n}} \Rightarrow b_e = b$ * NO REDUCTION IN A_g