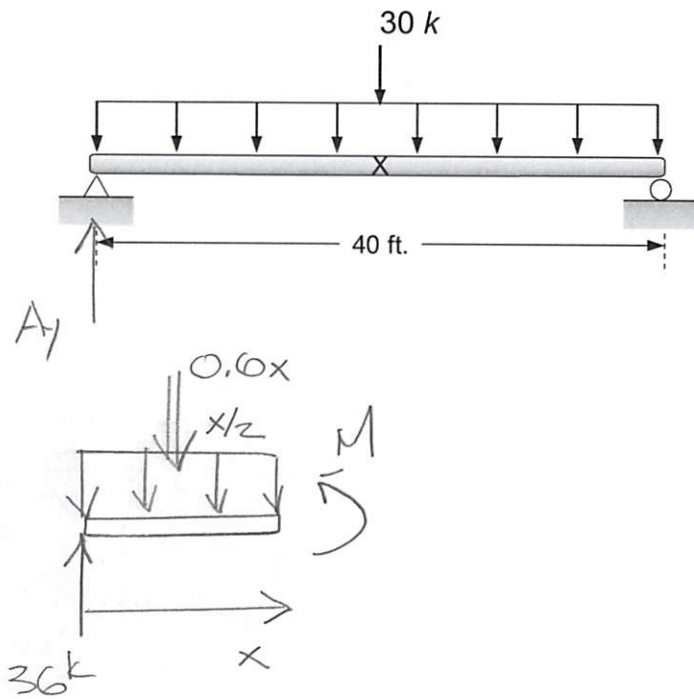


Classroom Problem 5.10-3: Select a **W** section of **A992** steel ($F_y = 50 \text{ ksi}$; $F = 65 \text{ ksi}$) for the following beam. The beam is laterally braced at the ends and at mid-span. The beam supports a uniform dead load of 0.5 k/ft and a live load of 30 k at the center of the span. There is no limit on deflection. Use Table 3-10 in the Manual.



$$W_D = 1.2(0.5 \text{ k/ft}) = 0.6 \text{ k/ft}$$

$$P_L = 1.6(30 \text{ k}) = 48 \text{ k}$$

$$\sum M_B = 0 = 0.6 \text{ k/ft}(40 \text{ ft})(20 \text{ ft}) + 48 \text{ k}(20 \text{ ft}) - A_1(40 \text{ ft})$$

$$A_1 = 36 \text{ k}$$

$$\sum M_{cut} = 0 = M + 0.6x\left(\frac{x}{2}\right) - 36x$$

$$M(x) = \underline{\underline{[-0.3x^2 + 36x] \text{ k ft}}}$$

$$M_A(x=5 \text{ ft}) = 172.5 \text{ k ft}$$

$$M_B(x=10 \text{ ft}) = 330 \text{ k ft}$$

$$M_c(x=15 \text{ ft}) = 540 \text{ k ft}$$

$$M_{MAX}(x=20 \text{ ft}) = 600 \text{ k ft}$$

$$C_b = \frac{12.5(600)}{2.5(600) + 3(172.5) + 4(330) + 3(540)} = 1.513$$

Classroom Problem 5.10-3: Select a **W** section of **A992** steel ($F_y = 50$ ksi; $F = 65$ ksi) for the following beam. The beam is laterally braced at the ends and at mid-span. The beam supports a uniform dead load of 0.5 k/ft and a live load of 30 k at the center of the span. There is no limit on deflection.

$$\frac{M_U}{C_b} = \frac{600 \text{ k}\cdot\text{ft}}{1.513} = 396 \text{ k}\cdot\text{ft} \text{ WITH } L_b = 20 \text{ ft}$$

FROM TABLE 3.10(3-119) W18x76

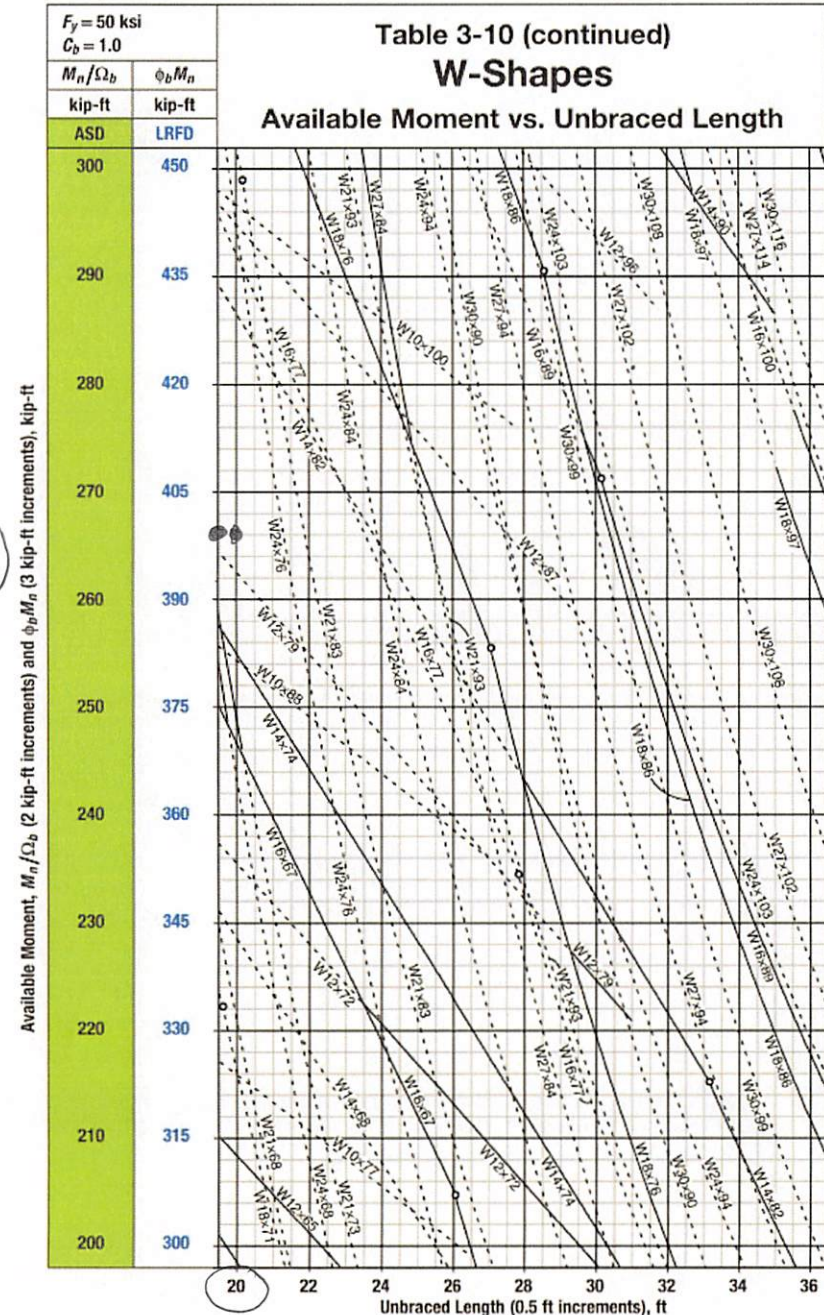
$$\phi M_n = C_b (474 \text{ k}\cdot\text{ft}) = 717.2 \text{ k}\cdot\text{ft}$$

(FROM TABLE 3.10(3-117))

$$\phi M_{n_{MAX}} = 612 \text{ k}\cdot\text{ft} \text{ FROM TABLE 3.10(3-114)}$$

$$\phi M_{n_{MAX}} = 612 \text{ k}\cdot\text{ft} > M_U = 600 \text{ k}\cdot\text{ft}$$

W18x24 o.k.



$$W_U = 1.2(0.5 \text{ k/ft} + 0.076 \text{ k/ft}) = 0.691 \text{ k/ft}$$

$$P_L = 1.6(30 \text{ k}) = 48 \text{ k}$$

$$M_U = \frac{W_U L^2}{8} + \frac{P_L L}{4} = \frac{(0.691 \text{ k/ft})(40 \text{ ft})^2}{8} + \frac{48 \text{ k}(40 \text{ ft})}{4} = 618.24 \text{ k}\cdot\text{ft}$$

$$\phi M_n = 613 \text{ k}\cdot\text{ft} < M_U = 618.24 \text{ k}\cdot\text{ft} \quad \underline{\text{N.G.}}$$

• TABLE 3.10 → NEXT CURVE TO RIGHT W18x86

$$\frac{M_U}{C_b} = \frac{618.24 \text{ k}\cdot\text{ft}}{1.513} = 409 \text{ k}\cdot\text{ft} \quad \text{WITH } L_b = 20 \text{ ft}$$

FROM TABLE 3.10 (1-114) → W18x86 → W27x84 $\phi_b M_n = 579 \text{ k}\cdot\text{ft}$ $C_b = 1$

$$\phi M_n = 1.513(579 \text{ k}\cdot\text{ft}) = 876 \text{ k}\cdot\text{ft} \quad \phi M_{n_{\text{MAX}}} = 915 \text{ k}\cdot\text{ft} > M_U = 618.24 \text{ k}\cdot\text{ft}$$

O.K.

USE W27x84