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### Chapter 5 – Compact Shapes

➤ Recall, the nominal bending strength for compact *I* and *C*-shaped sections can be summarized as follows:

For  $L_b \leq L_p$        $M_n = M_p$       *AISC Equation F2-1*

For  $L_p < L_b \leq L_r$        $M_n = C_b \left[ M_p - (M_p - 0.7F_y S_x) \left( \frac{L_b - L_p}{L_r - L_p} \right) \right] \leq M_p$       *AISC Equation F2-2*

For  $L_b > L_r$        $M_n = F_{cr} S_x \leq M_p$       *AISC Equation F2-3*

$$F_{cr} = \frac{C_b \lambda^2 E}{(L_b/r_{ts})^2} \sqrt{1 + 0.078 \frac{Jc}{S_x h_o} \left( \frac{L_b}{r_{ts}} \right)^2}$$
      *AISC Equation F2-4*

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### Chapter 5 – Compact Shapes

➤ Remember that  $C_b$  is a **lateral-torsional buckling modification factor** that accounts for nonuniform bending within the unbraced length  $L_b$ .

For  $L_b \leq L_p$        $M_n = M_p$       *AISC Equation F2-1*

For  $L_p < L_b \leq L_r$        $M_n = C_b \left[ M_p - (M_p - 0.7F_y S_x) \left( \frac{L_b - L_p}{L_r - L_p} \right) \right] \leq M_p$       *AISC Equation F2-2*

For  $L_b > L_r$        $M_n = F_{cr} S_x \leq M_p$       *AISC Equation F2-3*

$$F_{cr} = \frac{C_b \lambda^2 E}{(L_b/r_{ts})^2} \sqrt{1 + 0.078 \frac{Jc}{S_x h_o} \left( \frac{L_b}{r_{ts}} \right)^2}$$
      *AISC Equation F2-4*

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### Chapter 5 – Compact Shapes

➤ The  $C_b$  coefficient is used in flexural expressions to account for the **variation of bending moment** along the length of a member.

➤ End-supported beams or braced segments, either due to their loading arrangement and/or support restraint condition, can have **nonlinear bending moment diagrams**.

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### Chapter 5 – Compact Shapes

➤ The nominal flexural strength is **conservative** for beams with **nonlinear moment diagrams**.

➤ The  $C_b$  coefficient is used to more accurately model the actual strength of the beams.

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### Chapter 5 – Compact Shapes

➤ If there is a moment gradient, the value of  $C_b$  is given by:

$$C_b = \frac{12.5M_{max}}{2.5M_{max} + 3M_A + 4M_B + 3M_C}$$
      *AISC Equation F1-1*

where:  $M_{max}$  is the absolute value of the **maximum moment** within the unbraced length (including the end points of the unbraced length)

$M_A$  is the absolute value of the moment at the **quarter point** of the unbraced length

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### Chapter 5 – Compact Shapes

➤ If there is a moment gradient, the value of  $C_b$  is given by:

$$C_b = \frac{12.5M_{max}}{2.5M_{max} + 3M_A + 4M_B + 3M_C} \quad \text{AISC Equation F1-1}$$

where:  $M_B$  is the absolute value of the moment at the **midpoint** of the unbraced length

$M_C$  is the absolute value of the moment at the **three-quarter point** of the unbraced length

➤ **AISC Equation F1-1** is valid for doubly symmetric members and for singly symmetric members in single curvature.

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### Chapter 5 – Compact Shapes

➤ If there is a moment gradient, the value of  $C_b$  is given by:

$$C_b = \frac{12.5M_{max}}{2.5M_{max} + 3M_A + 4M_B + 3M_C} \quad \text{AISC Equation F1-1}$$

➤ When the bending moment is uniform,  $M$ , the value of  $C_b$  is:

$$C_b = \frac{12.5M}{2.5M + 3M + 4M + 3M} = 1.0$$

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### Chapter 5 – Compact Shapes

➤ **Example 5.5:** Determine  $C_b$  for a uniformly loaded, simply supported W shape with lateral support at its ends only.

Because of symmetry, the maximum moment is at midspan, so

$$M_{max} = M_B = \frac{wL^2}{8} \quad \text{AISC Table 3-22}$$

Also, due to symmetry, the moment at the quarter point equals the moment at the three-quarter point.

$$M_A = M_C = \frac{wL}{2} \left( \frac{L}{4} \right) - \frac{wL}{4} \left( \frac{1}{2} \right) \left( \frac{L}{4} \right) = \frac{3wL^2}{32}$$

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### Chapter 5 – Compact Shapes

➤ **Example 5.5:** Determine  $C_b$  for a uniformly loaded, simply supported W shape with lateral support at its ends only.

$$C_b = \frac{12.5M_{max}}{2.5M_{max} + 3M_A + 4M_B + 3M_C} = \frac{12.5(1/8)}{2.5(1/8) + 3(3/32) + 4(1/8) + 3(3/32)} = 1.136$$

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### Chapter 5 – Compact Shapes

➤ Values of  $C_b$  are tabulated for many common cases of loading and lateral support.

➤ These values can be found in **Table 3-1** in **Part 3 of the Manual**, "Design of Flexural Members."

Table 3-1 Values of $C_b$ for Simply Supported Beams		
Load	Lateral Bracing Along Span	$C_b$
	None	1.32
	At midpoint	1.32
	None	1.14
	At load points	1.14
	None	1.14
	At load points	1.14

Table 3-1 Values of $C_b$ for Simply Supported Beams		
Load	Lateral Bracing Along Span	$C_b$
	None	1.14
	At midpoint	1.14
	At third points	1.14, 1.01, 1.01, 1.14
	At quarter points	1.14, 1.06, 1.06, 1.14
	At fifth points	1.14, 1.17, 1.17, 1.14, 1.14

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### Chapter 5 – Compact Shapes

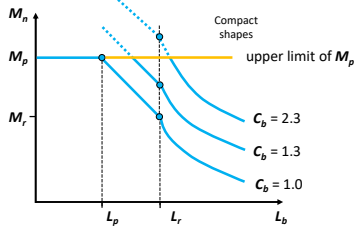
➤ Values of  $C_b$  are tabulated for many common cases of loading and lateral support.

➤ These values can be found in **Table 3-1** in **Part 3 of the Manual**, "Design of Flexural Members."


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### Chapter 5 – Compact Shapes

- The effect of  $C_b$  on the nominal strength is shown below.
- Although the strength is directly proportional to  $C_b$ , this graph clearly shows the importance of observing the upper limit of  $M_p$ , regardless of which equation is used for  $M_n$ .



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### Chapter 5 – Compact Shapes

- **Part 3 of the Steel Construction Manual**, “Design of Flexural Members,” contains several useful tables and charts for the analysis and design of beams.
- For example, **Table 3-2, “W Shapes, Selection by  $Z_x$ ”** (hereafter referred to as the “ $Z_x$  table”).
- The  $Z_x$  table lists shapes commonly used as beams, arranged in order of **available flexural strength**—  $\phi_b M_{px}$ .

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### Chapter 5 – Compact Shapes

Table 3-2 (continued)  
W-Shapes  
 $F_y = 50 \text{ ksi}$

Table 3-2 (continued)  
W-Shapes  
Selection by  $Z_x$

Shape	$Z_x$ in. <sup>3</sup>	$f_y/\Omega_c$		$\phi_b M_{px}$		$M_n/\Omega_c$		$\phi_b M_{rx}$		$BF/\Omega_c$		$\phi_b BF$		$L_p$ ft	$L_r$ ft	$L_c$ in. <sup>4</sup>	$V_{ux}/\Omega_c$		$\phi_v V_{ux}$	
		ASD	LRFD	kip-ft	kip-ft	kip-ft	kip-ft	ASD	LRFD	ASD	LRFD	kip	kip				ASD	LRFD		
W21x55	126	314	473	192	289	10.8	16.3	6.11	17.4	1140	156	234								
W14x74	126	314	473	196	294	5.31	8.05	8.76	31.0	785	128	192								
W18x60	123	307	461	189	284	9.62	14.4	5.93	18.2	964	151	227								
W12x79	119	297	446	187	281	3.78	5.67	10.8	39.9	662	117	175								
W14x68	115	287	431	180	270	5.19	7.81	8.69	29.3	722	116	174								
W10x88	113	282	424	172	259	2.62	3.94	9.29	51.2	534	131	196								
W18x55	112	279	420	172	258	9.15	13.8	5.90	17.6	890	141	212								
W21x50	110	274	413	165	248	12.1	18.3	4.59	13.6	984	158	237								
W12x72	108	269	405	170	255	3.89	5.56	10.7	37.5	597	106	159								
W21x48 <sup>†</sup>	107	265	398	162	244	9.88	14.8	6.89	16.5	959	144	216								
W16x57	105	262	394	161	242	7.88	12.0	5.65	18.3	758	141	212								
W14x61	102	254	383	161	242	4.93	7.48	8.65	27.5	640	104	156								
W18x50	101	252	379	155	233	8.76	13.2	5.83	16.9	800	129	192								
W10x77	97.6	244	366	150	225	2.69	3.90	9.18	45.3	455	112	169								
W12x65 <sup>††</sup>	96.8	237	356	154	231	3.58	5.39	11.9	35.1	532	94.4	142								

increasing values of  $Z_x$  ↑

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### Chapter 5 – Compact Shapes

- The  $Z_x$  table has tabulated values for  $L_p$  and  $L_r$  (which is particularly tedious to compute).
- Values for  $L_p$  and  $L_r$  can also be found in several other tables in **Part 3** of the Manual.
- We cover additional **design aids** in other sections of this chapter.

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### Chapter 5 – Compact Shapes

Let’s work on some problems



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### Chapter 5 – Beams

Any questions?



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