
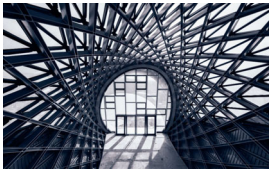


1

Chapter 5 – Design

- **Beam design** entails the *selection of a cross-sectional shape* that will have **enough strength** and that will meet **serviceability requirements**.
- As far as strength is concerned, **flexure** is almost always more critical than **shear**, so the usual practice is to **design for flexure and then check shear**.

2

Chapter 5 – Design

- The **design process** can be outlined as follows:
 1. Compute the required moment strength (i.e., the factored load moment M_u for **LRFD**).

The **weight of the beam** is part of the dead load but is unknown at this point.

Because the **beam weight** is usually a small part of the total load, it is ignored at the beginning of a design problem.

The selected shape will usually be **satisfactory** when the moment is recomputed with beam weight.

3

Chapter 5 – Design

- The **design process** can be outlined as follows:
 2. Compute the **moment of inertia required** to meet the **deflection limit**.

Use the **service load** for this computation.

 3. Select a **shape that meets the strength requirement**, then check the moment of inertia. Select a different shape if necessary.

4

Chapter 5 – Design

- The **design process** can be outlined as follows:
 4. Compute the required moment strength and moment of inertia using the **actual beam weight**.

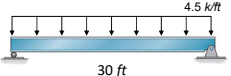
If necessary, select a different shape.

 5. Check the **shear strength**.- The trial shape can be easily selected in only a **limited** number of situations.
- In most cases, **design aids**, such as the **beam design charts** in **Part 3 of the Manual**, can be used.

5

Chapter 5 – Design

- **Example 5.10:** Select a standard hot-rolled shape of **A992** steel ($F_y = 50 \text{ ksi}$; $F = 65 \text{ ksi}$) for the beam shown. The beam has continuous lateral support and must support a uniform service live load of 4.5 kips/ft . The maximum permissible live load deflection is $L/240$.



LRFD Solution

Initially, ignore the beam weight.

$$w_u = 1.2w_D + 1.6w_L = 1.2(0) + 1.6(4.5 \text{ k/ft}) = 7.2 \text{ k ft}$$

$$M_u = \frac{w_u L^2}{8} = \frac{7.2 \text{ k/ft} (30 \text{ ft})^2}{8} = 810.0 \text{ k ft} = \phi_b M_n$$

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Chapter 5 – Design

➤ **Example 5.10:** The maximum permissible live load deflection is:

$$\frac{L}{240} = \frac{360in}{240} = 1.5in \leq \frac{5wL^4}{384EI}$$

$$\therefore I = \frac{5wL^4}{384E\Delta} = \frac{5(4.5k/ft)(1ft/12in)(360in)^4}{384(29,000ksi)(1.5in)} = 1,885.4in^4$$

Assume that the shape will be **compact**. For a compact shape with full lateral support:

$$M_n = M_p = F_y Z_x \quad \phi_b M_n > M_u \quad \therefore \phi_b F_y Z_x > M_u$$

$$Z_x > \frac{M_u}{\phi_b F_y} = \frac{810.0kft(12in/ft)}{0.90(50ksi)} = 216in^3$$

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Chapter 5 – Design

➤ **Example 5.10:** The Z_x table lists hot-rolled shapes commonly used as beams, ordered by **decreasing plastic section modulus**.

Table 3-2
W-Shapes
Selection by Z_x
 $F_y = 50$ ksi

Shape	Z_x in. ³	M_u/Ω_c		$\phi_b M_n$		M_u/Ω_c		$\phi_b M_n$		RF/Ω _c	φ _b RF	L _p ft	L _r ft	I _x in. ⁴	I _y in. ⁴	V _u /Ω _v	φ _v V _u
		kip-ft	kip-ft	kip-ft	kip-ft	kip-ft	kip-ft										
W36x420	4130	10300	15500	5920	8900	47.6	71.7	15.0	107	73000	2900	3900					
W36x400	3920	9780	14700	5680	8530	46.3	72.7	15.1	100	70000	2370	3260					
W36x380	3660	9130	13700	5310	7980	46.0	71.9	14.9	94.5	64800	2300	3040					
W36x360	3270	8160	12300	4790	7190	47.6	72.2	14.7	85.5	57300	1810	2720					
W40x650	3080	7680	11600	4520	6800	56.1	85.3	13.6	69.9	56500	1720	2580					
W40x630	2910	7260	10900	4300	6460	46.8	70.3	14.5	77.7	50600	1620	2430					
W40x610	2760	6890	10400	4090	6140	55.4	84.4	13.4	63.9	50400	1540	2310					
W36x520	2330	5810	8740	3480	5220	46.4	70.1	14.1	64.3	39600	1390	1920					
W40x520	2320	5790	8700	3460	5200	55.3	83.1	13.1	55.2	41600	1300	1960					
W36x480	2130	5310	7990	3200	4800	46.0	69.5	14.0	59.9	39000	1170	1720					
W14x87	2030	5060	7610	2670	4020	7.67	11.5	17.3	329	18100	1860	2790					

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Chapter 5 – Design

➤ **Example 5.10:** Also, they are grouped so that the shape at the top of each group (in **bold type**) is the lightest one that has enough section modulus to satisfy a required section modulus that falls within the group.

Table 3-2
W-Shapes
Selection by Z_x
 $F_y = 50$ ksi

Shape	Z_x in. ³	M_u/Ω_c		$\phi_b M_n$		M_u/Ω_c		$\phi_b M_n$		RF/Ω _c	φ _b RF	L _p ft	L _r ft	I _x in. ⁴	I _y in. ⁴	V _u /Ω _v	φ _v V _u
		kip-ft	kip-ft	kip-ft	kip-ft	kip-ft	kip-ft										
W36x420	4130	10300	15500	5920	8900	47.6	71.7	15.0	107	73000	2900	3900					
W36x400	3920	9780	14700	5680	8530	46.3	72.7	15.1	100	70000	2370	3260					
W36x380	3660	9130	13700	5310	7980	46.0	71.9	14.9	94.5	64800	2300	3040					
W36x360	3270	8160	12300	4790	7190	47.6	72.2	14.7	85.5	57300	1810	2720					
W40x650	3080	7680	11600	4520	6800	56.1	85.3	13.6	69.9	56500	1720	2580					
W40x630	2910	7260	10900	4300	6460	46.8	70.3	14.5	77.7	50600	1620	2430					
W40x610	2760	6890	10400	4090	6140	55.4	84.4	13.4	63.9	50400	1540	2310					
W36x520	2330	5810	8740	3480	5220	46.4	70.1	14.1	64.3	39600	1390	1920					
W40x520	2320	5790	8700	3460	5200	55.3	83.1	13.1	55.2	41600	1300	1960					
W36x480	2130	5310	7990	3200	4800	46.0	69.5	14.0	59.9	39000	1170	1720					
W14x87	2030	5060	7610	2670	4020	7.67	11.5	17.3	329	18100	1860	2790					

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Chapter 5 – Design

➤ **Example 5.10:** In this example, the shape that comes closest to meeting the section modulus requirement is **W21 x 93**

$$Z_x = 216in^3$$

Table 3-2 (continued)
W-Shapes
Selection by Z_x
 $F_y = 50$ ksi

Shape	Z_x in. ³	M_u/Ω_c		$\phi_b M_n$		M_u/Ω_c		$\phi_b M_n$		RF/Ω _c	φ _b RF	L _p ft	L _r ft	I _x in. ⁴	I _y in. ⁴	V _u /Ω _v	φ _v V _u
		kip-ft	kip-ft	kip-ft	kip-ft	kip-ft	kip-ft										
W21x93	221	551	829	335	504	14.6	22.0	6.50	21.3	2070	251	376					
W14x120	212	529	795	332	499	5.09	7.65	13.2	51.9	1380	171	257					
W18x97	211	526	791	328	494	9.41	14.1	9.36	30.4	1750	199	299					
W24x76	200	499	750	307	462	15.1	22.6	6.78	19.5	2100	210	315					
W18x100	198	494	743	306	459	7.86	11.9	8.87	32.8	1490	199	298					
W21x83	196	489	735	299	449	13.9	20.8	6.46	20.2	1830	220	331					
W14x109	192	479	720	302	454	5.01	7.54	13.2	48.5	1240	150	225					
W18x86	186	464	698	290	436	9.01	13.6	9.29	28.6	1530	177	265					
W12x120	186	464	698	290	436	3.94	5.95	11.1	56.5	1070	186	279					

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Chapter 5 – Design

➤ **Example 5.10:** However, there is one lighter.

$$Z_x = 216in^3$$

Table 3-2 (continued)
W-Shapes
Selection by Z_x
 $F_y = 50$ ksi

Shape	Z_x in. ³	M_u/Ω_c		$\phi_b M_n$		M_u/Ω_c		$\phi_b M_n$		RF/Ω _c	φ _b RF	L _p ft	L _r ft	I _x in. ⁴	I _y in. ⁴	V _u /Ω _v	φ _v V _u
		kip-ft	kip-ft	kip-ft	kip-ft	kip-ft	kip-ft										
W21x93	221	551	829	335	504	14.6	22.0	6.50	21.3	2070	251	376					
W14x120	212	529	795	332	499	5.09	7.65	13.2	51.9	1380	171	257					
W18x97	211	526	791	328	494	9.41	14.1	9.36	30.4	1750	199	299					
W24x76	200	499	750	307	462	15.1	22.6	6.78	19.5	2100	210	315					
W18x100	198	494	743	306	459	7.86	11.9	8.87	32.8	1490	199	298					
W21x83	196	489	735	299	449	13.9	20.8	6.46	20.2	1830	220	331					
W14x109	192	479	720	302	454	5.01	7.54	13.2	48.5	1240	150	225					
W18x86	186	464	698	290	436	9.01	13.6	9.29	28.6	1530	177	265					
W12x120	186	464	698	290	436	3.94	5.95	11.1	56.5	1070	186	279					

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Chapter 5 – Design

➤ **Example 5.10:** However, there is one lighter. **W24 x 84**

$$Z_x = 216in^3$$

Table 3-2 (continued)
W-Shapes
Selection by Z_x
 $F_y = 50$ ksi

Shape	Z_x in. ³	M_u/Ω_c		$\phi_b M_n$		M_u/Ω_c		$\phi_b M_n$		RF/Ω _c	φ _b RF	L _p ft	L _r ft	I _x in. ⁴	I _y in. ⁴	V _u /Ω _v	φ _v V _u
		kip-ft	kip-ft	kip-ft	kip-ft	kip-ft	kip-ft										
W24x84	224	559	840	342	515	16.2	24.2	6.89	20.3	2370	227	340					
W12x136	214	534	803	325	485	4.00	6.08	11.2	63.2	1240	212	318					
W14x120	212	529	795	332	499	5.09	7.65	13.2	51.9	1380	171	257					
W18x97	211	526	791	328	494	9.41	14.1	9.36	30.4	1750	199	299					
W24x76	200	499	750	307	462	15.1	22.6	6.78	19.5	2100	210	315					
W18x100	198	494	743	306	459	7.86	11.9	8.87	32.8	1490	199	298					
W21x83	196	489	735	299	449	13.9	20.8	6.46	20.2	1830	220	331					
W14x109	192	479	720	302	454	5.01	7.54	13.2	48.5	1240	150	225					
W18x86	186	464	698	290	436	9.01	13.6	9.29	28.6	1530	177	265					
W12x120	186	464	698	290	436	3.94	5.95	11.1	56.5	1070	186	279					

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Chapter 5 – Design

➤ **Example 5.10:** Try a **W24 x 84**: This shape is *compact*, as assumed (noncompact shapes are marked as such in the table); therefore $M_n = M_p$, as assumed.

From Table 1-1 (1-20): $I_x = 2,370 in^4 > 1,885 in^4$ **O.K.**

Account for the weight of the beam:

$$w_u = 1.2w_d + 1.6w_L = 1.2(0.084 k/ft) + 1.6(4.5 k/ft) = 7.301 k ft$$

$$M_u = \frac{w_u L^2}{8} = \frac{7.301 k/ft (30 ft)^2}{8} = 821.4 k ft$$

$$Z_x > \frac{M_u}{\phi_b F_y} = \frac{821.4 k ft (12 in/ft)}{0.90(50 ksi)} = 219.04 in^3 < 224 in^3$$
 O.K.

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Chapter 5 – Design

➤ **Example 5.10:** Instead of basing the search on the required section modulus, the design strength $\phi_b M_p$ could be used, because it is directly proportional to Z_x and is also tabulated.

$$\phi_b M_n = 810.0 k ft$$

Table 3-2 (continued)
W-Shapes
Selection by Z_x
 $F_y = 50 ksi$

Shape	Z_x in ³	M_n/Ω_c		$\phi_b M_n$		R_F/Ω_c	$\phi_b R_F$	L_p ft	L_r ft	I_x in ⁴	I_y/Ω_c	$\phi_b V_{nx}$	$\phi_b V_{ny}$
		kip-ft	kip-ft	kip-ft	kip-ft								
W24x84	224	162	940	142	810	16.2	24.2	6.89	20.3	2370	227	340	340
W12x136	214	534	803	525	488	4.02	6.06	11.2	63.2	1240	212	318	318
W14x120	212	559	795	532	499	5.08	7.65	12.2	51.9	1380	171	257	257
W18x97	211	526	791	528	494	9.41	14.1	9.36	30.4	1750	199	299	299
W24x76	200	499	750	507	462	18.1	22.6	6.78	19.5	2100	210	315	315
W16x100	198	484	743	506	459	7.36	11.9	8.87	52.8	1490	199	298	298
W21x83	196	489	735	509	459	13.9	20.8	6.46	20.2	1930	220	331	331
W14x109	192	479	720	502	454	5.01	7.54	13.2	48.5	1240	150	225	225
W18x89	186	464	698	500	436	6.01	13.6	9.29	28.6	1530	177	265	265
W12x120	186	464	698	525	426	3.94	5.95	11.1	56.5	1070	186	279	279

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Chapter 5 – Design

Let's work on some problems



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Chapter 5 – Beams

Any questions?



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