

Chapter 4 Compression Members

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- For isolated columns that are not part of a continuous frame, **Table C-A-7.1** in the *Commentary to Specification Appendix 7* will usually suffice.

	0.5	0.7	1.0	1.0	2.0	2.0
Theoretical K value	0.5	0.7	1.0	1.0	2.0	2.0
Practical design value when other conditions are approximated	0.65	0.8	1.0	1.2	2.1	2.0

End condition codes:

- Rotation fixed and translation fixed
- Rotation free and translation fixed
- Rotation fixed and translation free
- Rotation free and translation free
- Rotation fixed, horizontal translation fixed, and vertical translation free
- Rotation free, horizontal translation fixed, and vertical translation free

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- Consider, however, this rigid frame.
- The columns in this frame are not independent members but part of a continuous structure.

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- The columns are restrained at both ends by their connection to beams and other columns.
- This frame is also unbraced, meaning that **horizontal sideways** displacements are possible.

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- If **Table C-A-7.1** is used for this frame, the lower-story columns are best approximated by **condition (f)**, and a value of $K = 2$.

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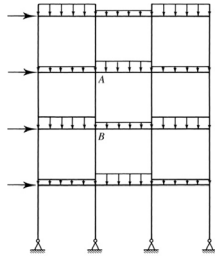
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- For a column such as AB, a value of $K = 1.2$, corresponding to **condition (d)**, could be selected.

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- A more rational procedure, however, will account for the degree of restraint provided by connecting members.

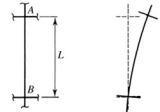


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- The rotational restraint provided by the beams, or girders, at the end of a column is a function of the **rotational stiffnesses** of the **members intersecting at the joint**.
- The rotational stiffness of a member is proportional to EI/L , where I is the moment of inertia of the cross section with respect to the axis of bending.
- The **effective length factor K** depends on the ratio of column stiffness to girder stiffness at each end of the member, which can be expressed as:

$$G = \frac{\sum E_{col} I_{col} / L_{col}}{\sum E_g I_g / L_g} = \frac{\sum I_{col} / L_{col}}{\sum I_g / L_g}$$

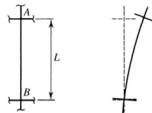


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- If a very **slender column** is connected to **girders having large cross-sections**, the girders will effectively prevent rotation of the column.
- The ends of the column are approximately **fixed**, and K is relatively small.
- This condition corresponds to small values of G .

$$G = \frac{\sum E_{col} I_{col} / L_{col}}{\sum E_g I_g / L_g} = \frac{\sum I_{col} / L_{col}}{\sum I_g / L_g}$$

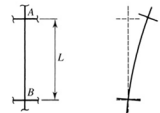


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- If **stiff columns** are connected to **flexible beams**, then the ends can more freely rotate.
- The column ends approaches the **pinned condition**, giving relatively large values of G and K

$$G = \frac{\sum E_{col} I_{col} / L_{col}}{\sum E_g I_g / L_g} = \frac{\sum I_{col} / L_{col}}{\sum I_g / L_g}$$



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- The relationship between G and K has been quantified in the Jackson–Mooreland Alignment Charts (Johnston, 1976), which are reproduced in **Table C-A-7.2 in the Commentary**.

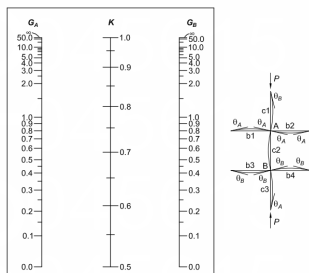


Fig. C-A-7.1. Alignment chart—sideways inhibited (braced frame).

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- The relationship between G and K has been quantified in the Jackson–Mooreland Alignment Charts (Johnston, 1976), which are reproduced in **Table C-A-7.2 in the Commentary**.

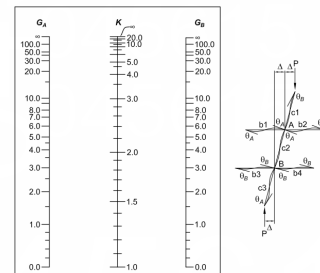


Fig. C-A-7.2. Alignment chart—sideways uninhibited (moment frame).

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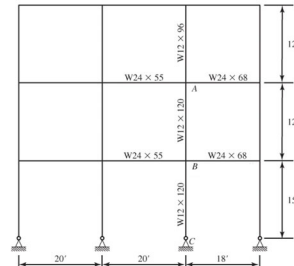
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- To obtain a value of K from one of these nomograms, first calculate the value of G .
- At each end of the column, let one value be G_A and the other be G_B .
- Connect G_A and G_B with a straight line and **read the value of on the middle scale.**
- The **effective length factor** obtained with respect to the axis of bending, which is the axis perpendicular to the plane of the frame.

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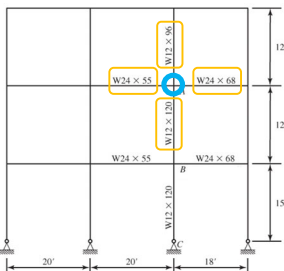
- **Example 4-13:** The rigid frame below is unbraced. Each member is oriented so that its web is in the plane of the frame. Determine the effective length factor K_x for columns AB and BC.



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- **Example 4-13:** Column AB, joint A:
 From Table 1-1 (1-20): $I_{W24 \times 55} = 1,350in^4$ $I_{W24 \times 68} = 1,830in^4$
 From Table 1-1 (1-26): $I_{W12 \times 120} = 1,070in^4$ $I_{W12 \times 96} = 833in^4$

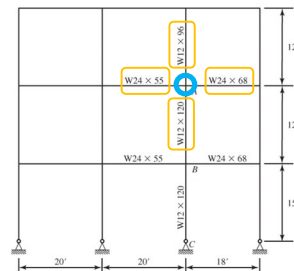


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- **Example 4-13:** Column AB, joint A:

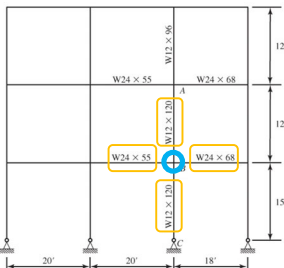
$$G_A = \frac{\sum I_{col} / L_{col}}{\sum I_g / L_g} = \frac{\sum (833in^4 / 12ft) + (1,070in^4 / 12ft)}{\sum (1,350in^4 / 20ft) + (1,830in^4 / 18ft)} = 0.937$$



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- **Example 4-13:** Column AB, joint B:
 From Table 1-1 (1-20): $I_{W24 \times 55} = 1,350in^4$ $I_{W24 \times 68} = 1,830in^4$
 From Table 1-1 (1-26): $I_{W12 \times 120} = 1,070in^4$

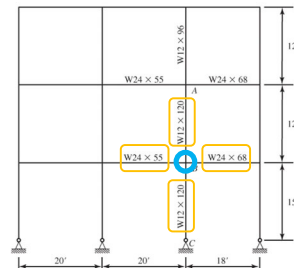


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- **Example 4-13:** Column AB, joint B:

$$G_B = \frac{\sum I_{col} / L_{col}}{\sum I_g / L_g} = \frac{\sum (1,070in^4 / 12ft) + (1,070in^4 / 15ft)}{\sum (1,350in^4 / 20ft) + (1,830in^4 / 18ft)} = 0.949$$



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➤ **Example 4-13:** From *AISC Figure C-A-7.2* for sidesway uninhibited, with $G_A = 0.937$ and $G_B = 0.949$.

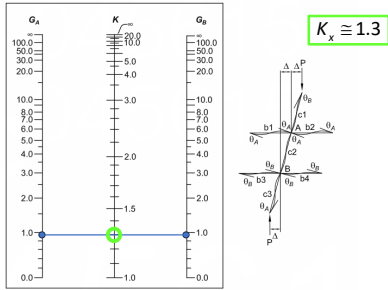


Fig. C-A-7.2. Alignment chart—sidesway uninhibited (moment frame).

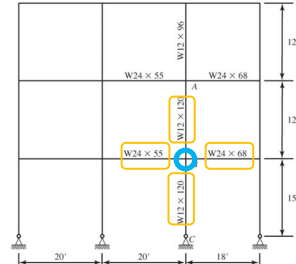
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➤ **Example 4-13:** Column *BC*, joint *B*:

From Table 1-1 (1-20): $I_{W24 \times 55} = 1,350in^4$ $I_{W24 \times 68} = 1,830in^4$

From Table 1-1 (1-26): $I_{W12 \times 120} = 1,070in^4$

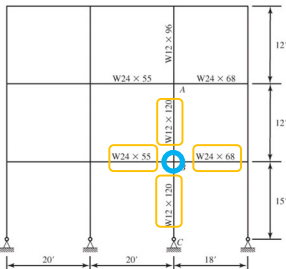


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➤ **Example 4-13:** Column *AB*, joint *B*:

$$G_B = \frac{\sum I_{col} / L_{col}}{\sum I_g / L_g} = \frac{\sum (1,070in^4 / 12ft) + (1,070in^4 / 15ft)}{\sum (1,350in^4 / 20ft) + (1,830in^4 / 18ft)} = 0.949$$

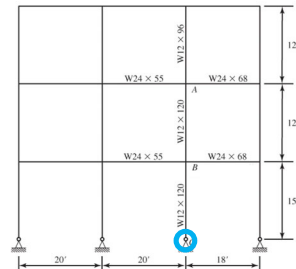


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➤ **Example 4-13:** Column *AB*, joint *C*:

For joint *C*, a pin connection, the situation is analogous to that of a very stiff column attached to infinitely flexible girders.

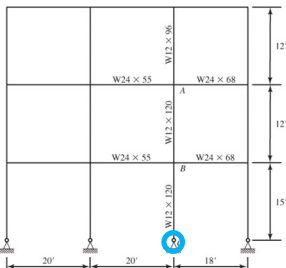


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➤ **Example 4-13:** Column *AB*, joint *C*:

The ratio of column stiffness to girder stiffness would therefore be infinite for a perfectly frictionless hinge.

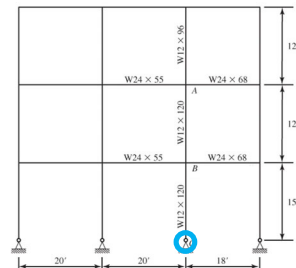


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➤ **Example 4-13:** Column *AB*, joint *C*:

This end condition can only be approximated in practice, so the discussion accompanying the alignment chart recommends that $G = 10.0$.



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➤ **Example 4-13:** From *AISC Figure C-A-7.2* for sidesway uninhibited, with $G_A = 0.949$ and $G_B = 10$.

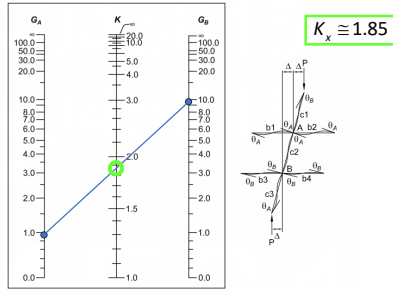


Fig. C-A-7.2. Alignment chart—sidesway uninhibited (moment frame).

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- As pointed out in **Example 4.13**, for a pinned support, G should be taken as 10.0; for a fixed support, G should be taken as 1.0.
- The latter support condition corresponds to an infinitely stiff girder and a flexible column, corresponding to a theoretical value of $G = 0$.
- The discussion accompanying the alignment chart in the **Commentary** recommends a value of $G = 1.0$ because true fixity will rarely be achieved.

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Let's work on some problems



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Any questions?



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