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Chapter 4 – More on Effective Length

- We introduced the concept of **effective length** in Section 4.2, “Column Theory.”
- All **compression members** are treated as pin-ended regardless of the actual end conditions, but with an effective length L_e that may differ from the actual length.
- With this modification, the load capacity of compression members is a function of only the **slenderness ratio** and **modulus of elasticity**.
- For a given material, the **load capacity** is a function of the **slenderness ratio** only.

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- If a **compression member** is supported differently with respect to each of its **principal axes**, the **effective length** will be different for the two directions.
- Consider a **W-shape** is used as a column and is braced by horizontal members in two perpendicular directions at the top.

(a) Minor Axis Buckling $L_e = 15'$ (b) Major Axis Buckling $L_e = 30'$

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- If a **compression member** is supported differently with respect to each of its **principal axes**, the **effective length** will be different for the two directions.
- These members **prevent translation** of the column in all directions, but the connections, permit **small rotations** to take place.

(a) Minor Axis Buckling $L_e = 15'$ (b) Major Axis Buckling $L_e = 30'$

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- Under these conditions, the member can be treated as **pin-connected** at the top and bottom.
- A **rigid, or fixed, condition is very difficult** to achieve, and ordinary connections will usually closely approximate a **hinge or pin connection**.

(a) Minor Axis Buckling $L_e = 15'$ (b) Major Axis Buckling $L_e = 30'$

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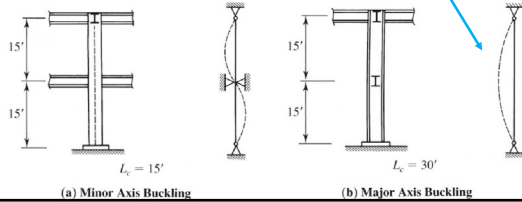
- At midheight, the column is **braced, but only in one direction**.
- This connection **prevents translation**, but **no restraint against rotation** is furnished.

(a) Minor Axis Buckling $L_e = 15'$ (b) Major Axis Buckling $L_e = 30'$

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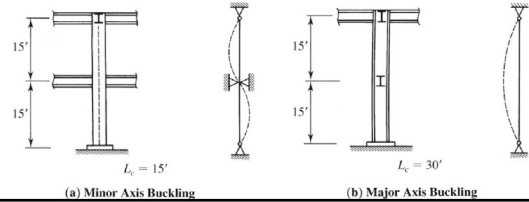
- This brace prevents translation perpendicular to the **weak axis of the cross section** but provides no restraint perpendicular to the strong axis.
- If the member were to buckle **about the major axis**, the **effective length** would be 30 ft.



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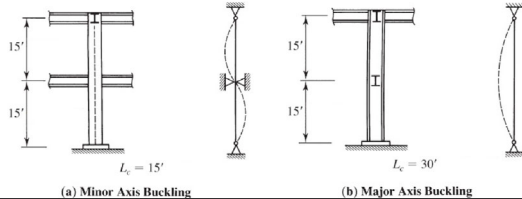
- Buckling about the **minor axis** would have to be in the **second buckling mode**, corresponding to an effective length of 15 ft.
- Because its strength decreases with increasing L_c/r , a column will buckle in the direction corresponding to the largest **slenderness ratio**.



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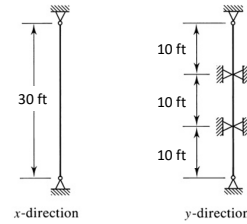
- Therefore, L_{cx}/r_x must be compared **with** L_{cy}/r_y
- In this example, the ratio $30\text{ ft}(12\text{ in/ft})/r_x$ must be compared with $15\text{ ft}(12\text{ in/ft})/r_y$
- The **largest ratio** would be used for the computing the **axial compressive strength**.



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- **Example 4-10:** A **W12 x 58**, 30 ft long, is pinned at both ends and braced in the weak direction at the third points. **A992** steel ($F_y = 50\text{ ksi}$) is used. Determine the available compressive strength.



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- **Example 4-10:** A **W12 x 58**, 30 ft long, is pinned at both ends and braced in the weak direction at the third points. **A992** steel ($F_y = 50\text{ ksi}$) is used. Determine the available compressive strength.

Table 1-1 (continued)
W-Shapes
Properties

Nominal W	Compact Section Criteria		Axis X-X				Axis Y-Y				r_x	r_y	Torsional Properties		
	d_b	t_w	I	S	r	Z	I	S	r	Z			J	C_w	
72	8.59	22.8	597	97.4	5.31	108	155	32.4	13.04	48.2	34.1	111.6	2.53	6560	
65	8.92	24.9	533	87.9	5.28	96.8	174	29.1	13.02	44.1	33.8	115.5	2.18	5790	
58	7.82	27.0	475	78.0	5.28	86.4	107	21.4	12.51	32.5	28.1	111.6	2.10	3570	
50	6.69	18.1	405	70.0	5.09	77.9	95.0	19.4	12.49	28.4	27.9	115.5	1.90	3100	
50	6.31	20.8	391	64.2	5.18	71.9	96.3	13.9	11.96	21.3	22.5	111.6	1.71	1880	
45	7.00	29.6	348	57.7	5.15	64.2	90.0	12.4	11.96	19.8	22.3	115.5	1.36	1690	
40	7.77	33.6	307	51.5	5.13	57.0	44.1	11.0	11.94	16.8	22.1	114.0	0.906	1440	
35	6.31	36.2	285	45.6	5.25	51.2	24.5	7.47	11.54	11.5	17.9	120.0	0.741	879	
30	7.41	41.8	258	38.6	5.21	43.1	20.3	6.24	11.52	9.50	17.7	111.9	0.457	720	
26	6.54	47.2	204	33.4	5.17	37.2	17.3	5.34	11.51	8.17	17.5	111.8	0.300	607	

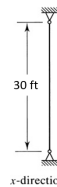
$r_x = 5.28\text{ in}$

$r_y = 2.51\text{ in}$

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- **Example 4-10:** A **W12 x 58**, 30 ft long, is pinned at both ends and braced in the weak direction at the third points. **A992** steel ($F_y = 50\text{ ksi}$) is used. Determine the available compressive strength.



$r_x = 5.28\text{ in}$

$r_y = 2.51\text{ in}$

$$\frac{L_{cx}}{r_x} = \frac{30\text{ ft}(12\text{ in/ft})}{5.28\text{ in}} = 68.18$$

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➤ **Example 4-10:** A **W12 x 58**, 30 ft long, is pinned at both ends and braced in the weak direction at the third points. **A992** steel ($F_y = 50$ ksi) is used. Determine the available compressive strength.

$r_x = 5.28in$
 $r_y = 2.51in$

$$\frac{L_{cx}}{r_x} = \frac{30ft(12in/ft)}{5.28in} = 68.18$$

$$\frac{L_{cy}}{r_y} = \frac{10ft(12in/ft)}{2.51in} = 47.81$$

$\frac{L_{cx}}{r_x}$ is the larger value and controls.

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➤ **Example 4-10:** A **W12 x 58**, 30 ft long, is pinned at both ends and braced in the weak direction at the third points. **A992** steel ($F_y = 50$ ksi) is used. Determine the available compressive strength.

➤ **LRFD** solution: Use **Table 4-14** with $KL/r = 68.18$ and interpolation values.

Also, $A_g = 17.0 in^2$ from Table 1-1 (1-26)

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$$32.1 + (31.8 - 32.1)0.18 = 32.05ksi$$

$$\phi_c F_{cr} = 32.05ksi$$

$$\phi_c P_n = \phi_c F_{cr} A_g = (32.05ksi)17.0in^2 = 544.8k$$

Table 4-14 (continued)

L_c/r	$F_y = 35$ ksi		$F_y = 40$ ksi		$F_y = 46$ ksi		$F_y = 50$ ksi		$F_y = 58$ ksi	
	ASD	LRFD	ASD	LRFD	ASD	LRFD	ASD	LRFD	ASD	LRFD
68	16.5	24.3	16.9	25.4	20.2	30.1	21	32.1	17.7	26.1
69	16.4	24.2	16.8	25.2	20.0	30.1	21	31.8	17.5	25.7

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➤ The available strengths given in the column load tables are based on the effective length with respect to the **y**-axis.

➤ A procedure for using the tables with L_{cx} can be developed by examining how the tabular values were obtained.

➤ Starting with a value of L_c , the strength was obtained by a procedure similar to the following:

- L_c was divided by r_y to obtain L_c/r_y
- F_n was computed
- The available strength $\phi_c P_n$ for **LRFD** and P_n/Ω_c for **ASD** were computed

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➤ The tabulated strengths are based on the values of L_c being equal to L_{cy} .

➤ If the capacity with respect to the x-axis buckling is desired, the table can be entered with:

$$L_c = \frac{L_{cx}}{r_x/r_y}$$

➤ The tabulated load will be based on:

$$\frac{L_c}{r_y} = \frac{L_{cx}/(r_x/r_y)}{r_y} = \frac{L_{cx}}{r_x}$$

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➤ The ratio r_x/r_y is given in the column load tables for each shape listed.

Table 4-1a Available Strength in Axial Compression, kips $F_y = 50$ ksi

Shape	ASD		LRFD		r_x/r_y
	$\phi_c P_n$	P_n/Ω_c	$\phi_c P_n$	P_n/Ω_c	
W12 x 58	17.7	26.1	21.0	32.1	1.9

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- The ratio r_x/r_y is given in the column load tables for each shape listed.

Table 4-1a
Available Strength in Axial Compression, kips
W-Shapes
 $F_y = 50$ ksi

Shape	67		58		48		40		35		31	
	P_n	$\phi_c P_n$	P_n	$\phi_c P_n$	P_n	$\phi_c P_n$	P_n	$\phi_c P_n$	P_n	$\phi_c P_n$	P_n	$\phi_c P_n$
0	590	886	512	769	422	634	350	526	308	463	273	411
6	542	815	470	706	387	581	300	481	281	423	249	374
7	526	790	453	685	383	569	299	465	272	409	241	362
9	488	733	422	634	321	485	225	343	211	317	222	333
11	444	668	384	576	314	473	258	388	226	340	200	301
12	421	633	363	546	297	447	243	366	213	321	199	293

Available Strength Parameters for Concentrated Forces¹

P_n , kips	6010	6010	5997	5940	5825	4230	2410	3020
P_n , kip/in.	131	107	125	107	102	154	84.3	142
P_n , kip/ft.	8350	14830	7860	12000	4620	8130	3482	9700
P_n , kip/s.	5080	8540	4910	7370	4510	6790	3820	5750

Properties

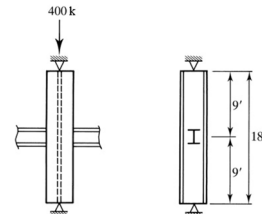
L_c , ft	17.3	17.1	16.6	16.3
L_c , in.	208	206	200	196
A_g , in ²	19100	15000	14300	12400
I_x , in ⁴	6170	5550	4720	4170
r_x/r_y	1.71	1.69	1.74	1.73
$P_n(L_c/10^3)$, kip-in ²	177000	159000	135000	119000

ASD LFRD
 $\Omega_c = 1.87$ $\phi_c = 0.90$

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- Example 4-11:** The compression member shown below is pinned at both ends and supported in the weak direction at midheight. A service load of 400 k, with equal parts dead and live load, must be supported. Use $F_y = 50$ ksi and select the lightest W-shape.



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- Example 4-11: LFRD solution:**

Factored Load = $1.2(200k) + 1.6(200k) = 560k$

Assume that the weak direction controls and enter **Table 4-1a** for $F_y = 50$ ksi and $L_c = 9$ ft.

Table 4-1a (continued)
Available Strength in Axial Compression, kips
W-Shapes
 $F_y = 50$ ksi

Shape	67		58		48		40		35		31	
	P_n	$\phi_c P_n$	P_n	$\phi_c P_n$	P_n	$\phi_c P_n$	P_n	$\phi_c P_n$	P_n	$\phi_c P_n$	P_n	$\phi_c P_n$
0	590	886	512	769	422	634	350	526	308	463	273	411
6	542	815	470	706	387	581	300	481	281	423	249	374
7	526	790	453	685	383	569	299	465	272	409	241	362
9	488	733	422	634	321	485	225	343	211	317	222	333
11	444	668	384	576	314	473	258	388	226	340	200	301
12	421	633	363	546	297	447	243	366	213	321	199	293

$\phi_c P_n = 634k$

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- Example 4-11:** From **Table 1-1** (1-28) a **W8 x 58** has:

$r_x = 3.65$ in $r_y = 2.10$ in $A_g = 17.1$ in²

Check the strong axis: $\frac{L_{cx}}{r_x/r_y} = \frac{18 \text{ ft}}{3.65 \text{ in} / 2.10 \text{ in}} = 10.36 \text{ ft} > 9 \text{ ft}$

Therefore, L_{cx} controls for this shape.

Enter the table with $L_c = 10.34$ ft.

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- Example 4-11:**

$606 + (576 - 606)0.34 = 594.8k$

$\phi_c P_n = 594.8k > 560k$ **OK**

Table 4-1a (continued)
Available Strength in Axial Compression, kips
W-Shapes
 $F_y = 50$ ksi

Shape	67		58		48		40		35		31	
	P_n	$\phi_c P_n$	P_n	$\phi_c P_n$	P_n	$\phi_c P_n$	P_n	$\phi_c P_n$	P_n	$\phi_c P_n$	P_n	$\phi_c P_n$
0	590	886	512	769	422	634	350	526	308	463	273	411
6	542	815	470	706	387	581	300	481	281	423	249	374
7	526	790	453	685	383	569	299	465	272	409	241	362
8	508	763	436	660	361	543	286	448	262	394	232	348
10	467	701	403	608	307	472	239	359	230	331	217	317
11	444	668	384	576	314	473	258	388	226	340	200	301
12	421	633	363	546	297	447	243	366	213	321	199	293

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- Example 4-11:** Try a **W10** shape

Factored Load = $1.2(200k) + 1.6(200k) = 560k$

Assume that the weak direction controls and enter **Table 4-1a** for $F_y = 50$ ksi and $L_c = 9$ ft.

Table 4-1a (continued)
Available Strength in Axial Compression, kips
W-Shapes
 $F_y = 50$ ksi

Shape	54		49		45		39		33	
	P_n	$\phi_c P_n$	P_n	$\phi_c P_n$	P_n	$\phi_c P_n$	P_n	$\phi_c P_n$	P_n	$\phi_c P_n$
0	673	1011	631	948	508	768	344	517	291	437
6	646	971	607	911	483	726	326	491	273	411
7	627	957	590	890	468	707	312	472	263	395
9	615	924	576	868	452	682	297	446	252	381
11	589	885	554	832	431	651	281	423	249	374
12	573	864	541	812	414	626	266	398	239	359
13	561	842	527	792	400	605	252	377	229	343
14	545	819	512	771	388	587	238	358	219	327

$\phi_c P_n = 568k$

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➤ Example 4-11: From Table 1-1 (1-28) a W10 x 49 has:

$$r_x = 4.35in \quad r_y = 2.54in \quad A_g = 14.4in$$

Check the strong axis: $\frac{L_{cx}}{r_x/r_y} = \frac{18ft}{4.35in/2.54in} = 10.51ft > 9ft$

Therefore, L_{cx} controls for this shape.

Enter the table with $L_c = 10.51ft$.

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➤ Example 4-11:

$$605 + (585 - 605)0.51 = 594.8k$$

$$\phi_c P_n = 594.8k > 560k \quad \text{OK}$$

Table 4-1a (continued)
Available Strength in Axial Compression, kips
W-Shapes
W10

Shape	W10							
	54		49		45		39	
Depth, h	ASD		ASD		ASD		ASD	
	P_n/Ω_c	$\phi_c P_n$	P_n/Ω_c	$\phi_c P_n$	P_n/Ω_c	$\phi_c P_n$	P_n/Ω_c	$\phi_c P_n$
0	473	711	431	648	398	598	344	517
6	486	671	447	611	383	565	313	470
7	437	657	388	588	330	527	302	454
8	427	642	388	584	327	507	300	436
10	401	605	349	550	307	461	283	396
11	389	585	34	532	291	437	249	374
12	361	541	327	498	256	385	219	329
13	345	519	313	471	239	359	203	306
14	329	497	299	446	223	334	187	282

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➤ Example 4-11: Try a W12 shape

$$\text{Factored Load} = 1.2(200k) + 1.6(200k) = 560k$$

Assume that the weak direction controls and enter Table 4-1a for $F_y = 50$ ksi and $L_c = 9ft$.

Table 4-1a (continued)
Available Strength in Axial Compression, kips
W-Shapes
W12

Shape	W12							
	58		53		45		40	
Depth, h	ASD		ASD		ASD		ASD	
	P_n/Ω_c	$\phi_c P_n$	P_n/Ω_c	$\phi_c P_n$	P_n/Ω_c	$\phi_c P_n$	P_n/Ω_c	$\phi_c P_n$
0	509	763	467	702	437	657	362	549
6	479	729	429	669	396	606	324	517
7	469	705	429	646	382	574	312	495
9	445	668	407	611	358	533	291	458
11	416	625	383	571	334	492	269	420
12	400	601	365	549	305	443	263	396
13	384	577	350	526	275	413	246	369
14	367	551	334	502	255	384	228	343
15	351	526	318	478	236	355	211	317

$$\phi_c P_n = 611k$$

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➤ Example 4-11: From Table 1-1 (1-26) a W12 x 53 has:

$$r_x = 5.23in \quad r_y = 2.48in \quad A_g = 15.6in$$

Check the strong axis: $\frac{L_{cx}}{r_x/r_y} = \frac{18ft}{5.23in/2.48in} = 8.54ft < 9ft$

Therefore, L_{cy} controls for this shape. $\phi_c P_n = 611k$

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➤ Example 4-11: Try a W14 shape

$$\text{Factored Load} = 1.2(200k) + 1.6(200k) = 560k$$

Assume that the weak direction controls and enter Table 4-1a for $F_y = 50$ ksi and $L_c = 9ft$.

Table 4-1a (continued)
Available Strength in Axial Compression, kips
W-Shapes
W14

Shape	W14							
	82		74		61		53	
Depth, h	ASD		ASD		ASD		ASD	
	P_n/Ω_c	$\phi_c P_n$	P_n/Ω_c	$\phi_c P_n$	P_n/Ω_c	$\phi_c P_n$	P_n/Ω_c	$\phi_c P_n$
0	719	1089	683	1019	500	800	467	702
6	676	1024	652	962	465	736	421	633
7	661	993	630	930	452	706	405	607
9	626	940	598	884	430	702	382	567
11	584	878	551	797	405	653	351	517
12	562	844	533	767	385	626	333	493
13	538	809	509	735	361	591	313	467
14	514	773	487	701	339	557	293	441

$$\phi_c P_n = 699k$$

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➤ Example 4-11: Try a W14 shape

$$\text{Factored Load} = 1.2(200k) + 1.6(200k) = 560k$$

The W14 x 61 is the lightest section possible, but it is heavier than the other sections.

Table 4-1a (continued)
Available Strength in Axial Compression, kips
W-Shapes
W14

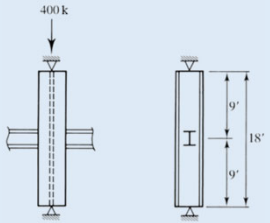
Shape	W14							
	82		74		61		53	
Depth, h	ASD		ASD		ASD		ASD	
	P_n/Ω_c	$\phi_c P_n$	P_n/Ω_c	$\phi_c P_n$	P_n/Ω_c	$\phi_c P_n$	P_n/Ω_c	$\phi_c P_n$
0	719	1089	683	1019	500	800	467	702
6	676	1024	652	962	465	736	421	633
7	661	993	630	930	452	706	405	607
9	626	940	598	884	430	702	382	567
11	584	878	551	797	405	653	351	517
12	562	844	533	767	385	626	333	493
13	538	809	509	735	361	591	313	467
14	514	773	487	701	339	557	293	441

$$\phi_c P_n = 699k$$

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➤ **Example 4-11:** The compression member shown below is pinned at both ends and supported in the weak direction at midheight. A service load of 400 k, with equal parts dead and live load, must be supported. Use $F_y = 50 \text{ ksi}$ and select the lightest **W-shape**.




W8 x 58
W10 x 54
W12 x 53 ←
W14 x 61

Use a **W12 x 53**

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Let's work on some problems



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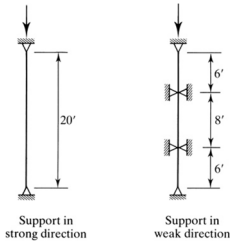
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- Whenever possible, the designer should provide extra support for the weak direction of a column.
- Otherwise, the member is **inefficient** (i.e., it has an excess of strength in one direction).
- When L_{cx} and L_{cy} are different, L_{cy} will control unless r_x/r_y is smaller than L_{cx}/L_{cy} .
- When the **two ratios are equal**, the column has **equal strength in both directions**.
- For most of the **W-shapes** in the column load tables, r_x/r_y ranges between 1.6 and 1.8, but it is as high as 3.1 for some shapes.

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➤ **Example 4-12:** The column shown below is subjected to a service dead load of 140 k and a service live load of 420 k. Use **A992 steel** ($F_y = 50 \text{ ksi}$) and select a **W-shape**.



$L_{cx} = 20 \text{ ft}$ $L_{cy} = 8 \text{ ft}$

The effective length L_{cx} will control whenever:

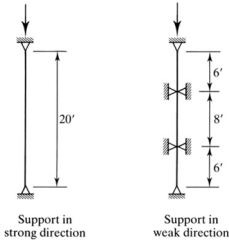
$$\frac{L_{cx}}{r_x/r_y} > L_{cy} \quad r_x/r_y < \frac{L_{cx}}{L_{cy}}$$

$$\frac{L_{cx}}{L_{cy}} = \frac{20 \text{ ft}}{8 \text{ ft}} = 2.5$$

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➤ **Example 4-12:** The column shown below is subjected to a service dead load of 140 k and a service live load of 420 k. Use **A992 steel** ($F_y = 50 \text{ ksi}$) and select a **W-shape**.



$L_{cx} = 20 \text{ ft}$ $L_{cy} = 8 \text{ ft}$

So, L_{cx} will control if $r_x/r_y < 2.5$

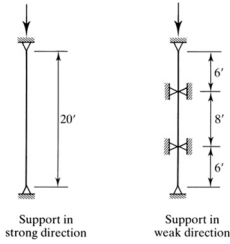
$$\frac{L_{cx}}{r_x/r_y} > L_{cy} \quad r_x/r_y < \frac{L_{cx}}{L_{cy}}$$

$$\frac{L_{cx}}{L_{cy}} = \frac{20 \text{ ft}}{8 \text{ ft}} = 2.5$$

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Chapter 4 – More on Effective Length

➤ **Example 4-12:** The column shown below is subjected to a service dead load of 140 k and a service live load of 420 k. Use **A992 steel** ($F_y = 50 \text{ ksi}$) and select a **W-shape**.



$L_{cx} = 20 \text{ ft}$ $L_{cy} = 8 \text{ ft}$

Since this is true for almost every shape in the column load tables, L_{cx} probably controls in this example.

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Chapter 4 – More on Effective Length

➤ **Example 4-12:** The column shown below is subjected to a service dead load of 140 k and a service live load of 420 k. Use **A992 steel** ($F_y = 50 \text{ ksi}$) and select a **W-shape**.

Assume $r_x/r_y = 1.7$

$$\frac{L_{cx}}{r_x/r_y} = \frac{20 \text{ ft}}{1.7} = 11.76 > L_{cy} = 8 \text{ ft}$$

Assume L_{cx} controls in this example.

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Chapter 4 – More on Effective Length

➤ **Example 4-12:** The column shown below is subjected to a service dead load of 140 k and a service live load of 420 k. Use **A992 steel** ($F_y = 50 \text{ ksi}$) and select a **W-shape**.

LRFD solution: $P_u = 1.2D + 1.6L = 1.2(140k) + 1.6(420k) = 840k$

Enter the column load table with $L_{cx} = 12 \text{ ft}$.

Assume that the strong direction controls and enter **Table 4-1a** for $F_y = 50 \text{ ksi}$ and $L_c = 12 \text{ ft}$.

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Chapter 4 – More on Effective Length

➤ **Example 4-12:** Try a **W8** shape for a length > 840 k

Assume that the strong direction controls and enter **Table 4-1a** for $F_y = 50 \text{ ksi}$ and $L_c = 12 \text{ ft}$.

There are **no W8 sections** with enough strength

Shape lb/ft	W8x											
	67		58		48		40		35		31	
	ASD	LRFD	ASD	LRFD	ASD	LRFD	ASD	LRFD	ASD	LRFD	ASD	LRFD
Design	P_n/ϕ_c	$\phi_c P_n$	P_n/ϕ_c	$\phi_c P_n$	P_n/ϕ_c	$\phi_c P_n$	P_n/ϕ_c	$\phi_c P_n$	P_n/ϕ_c	$\phi_c P_n$	P_n/ϕ_c	$\phi_c P_n$
0	590	866	512	769	422	634	350	526	463	273	411	611
6	562	815	470	706	387	581	320	481	393	432	389	574
7	526	790	455	683	375	563	309	465	372	409	341	502
8	509	763	438	660	361	543	298	448	362	394	332	486
9	488	733	422	634	347	521	285	429	351	377	322	453
10	467	701	403	606	331	497	272	409	339	359	311	437
12	421	633	363	546	297	447	243	366	313	321	189	283
14	373	560	321	482	262	394	213	321	187	281	155	248
15	358	523	299	450	244	367	199	298	174	261	139	230

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➤ **Example 4-12:** Try a **W10** shape for a length > 840 k

Assume that the strong direction controls and enter **Table 4-1a** for $F_y = 50 \text{ ksi}$ and $L_c = 12 \text{ ft}$.

Shape lb/ft	W10x											
	112		100		88		77		68		60	
	ASD	LRFD	ASD	LRFD	ASD	LRFD	ASD	LRFD	ASD	LRFD	ASD	LRFD
Design	P_n/ϕ_c	$\phi_c P_n$	P_n/ϕ_c	$\phi_c P_n$	P_n/ϕ_c	$\phi_c P_n$	P_n/ϕ_c	$\phi_c P_n$	P_n/ϕ_c	$\phi_c P_n$	P_n/ϕ_c	$\phi_c P_n$
0	985	1480	877	1320	778	1170	680	1000	596	895	530	796
6	934	1400	831	1250	737	1110	643	966	563	846	500	732
7	917	1380	815	1230	722	1090	630	946	552	829	490	717
8	897	1350	797	1200	706	1060	615	925	539	810	479	719
9	875	1310	777	1170	688	1030	599	900	525	789	466	700
10	851	1280	755	1130	669	1000	582	874	509	769	452	679
12	768	1100	707	1000	616	940	543	816	475	714	421	633
14	720	1110	654	983	578	868	501	753	438	658	388	583
15	708	1080	626	951	553	831	471	720	419	629	370	556

$\phi_c P_n = 940k$

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➤ **Example 4-12:** From Table 1-1 (1-28) a **W10 x 88** has:

$$r_x = 4.54 \text{ in} \quad r_y = 2.63 \text{ in} \quad A_g = 26.0 \text{ in}^2$$

$$r_x/r_y = \frac{4.60 \text{ in}}{2.63 \text{ in}} = 1.73$$

Shape lb/ft	W10x											
	112		100		88		77		68		60	
	ASD	LRFD	ASD	LRFD	ASD	LRFD	ASD	LRFD	ASD	LRFD	ASD	LRFD
Design	P_n/ϕ_c	$\phi_c P_n$	P_n/ϕ_c	$\phi_c P_n$	P_n/ϕ_c	$\phi_c P_n$	P_n/ϕ_c	$\phi_c P_n$	P_n/ϕ_c	$\phi_c P_n$	P_n/ϕ_c	$\phi_c P_n$
0	985	1480	877	1320	778	1170	680	1000	596	895	530	796
6	934	1400	831	1250	737	1110	643	966	563	846	500	732
7	917	1380	815	1230	722	1090	630	946	552	829	490	717
8	897	1350	797	1200	706	1060	615	925	539	810	479	719
9	875	1310	777	1170	688	1030	599	900	525	789	466	700
10	851	1280	755	1130	669	1000	582	874	509	769	452	679
12	768	1100	707	1000	616	940	543	816	475	714	421	633
14	720	1110	654	983	578	868	501	753	438	658	388	583
15	708	1080	626	951	553	831	471	720	419	629	370	556

$\phi_c P_n = 940k$

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➤ **Example 4-12:** From Table 1-1 (1-28) a **W10 x 88** has:

$$\text{Actual } \frac{L_{cx}}{r_x/r_y} = \frac{20 \text{ ft}}{1.73} = 11.56 \text{ ft} < 12 \text{ ft}$$

Shape lb/ft	W10x											
	112		100		88		77		68		60	
	ASD	LRFD	ASD	LRFD	ASD	LRFD	ASD	LRFD	ASD	LRFD	ASD	LRFD
Design	P_n/ϕ_c	$\phi_c P_n$	P_n/ϕ_c	$\phi_c P_n$	P_n/ϕ_c	$\phi_c P_n$	P_n/ϕ_c	$\phi_c P_n$	P_n/ϕ_c	$\phi_c P_n$	P_n/ϕ_c	$\phi_c P_n$
0	985	1480	877	1320	778	1170	680	1000	596	895	530	796
6	934	1400	831	1250	737	1110	643	966	563	846	500	732
7	917	1380	815	1230	722	1090	630	946	552	829	490	717
8	897	1350	797	1200	706	1060	615	925	539	810	479	719
9	875	1310	777	1170	688	1030	599	900	525	789	466	700
10	851	1280	755	1130	669	1000	582	874	509	769	452	679
12	768	1100	707	1000	616	940	543	816	475	714	421	633
14	720	1110	654	983	578	868	501	753	438	658	388	583
15	708	1080	626	951	553	831	471	720	419	629	370	556

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Example 4-12: By interpolation, a W10 x 88 has:

$$973 + (940 - 973)0.56 = 954.5k$$

$$\phi_c P_n = 954.5k > 840k \quad \text{OK}$$

Table 4-1a (continued)
Available Strength in Axial Compression, kips
W-Shapes
F_y = 50 ksi

Shape lb/ft	W10											
	112		100		88		77		68		60	
Design	P _n /1.2	φ _c P _n	P _n /1.2	φ _c P _n	P _n /1.2	φ _c P _n	P _n /1.2	φ _c P _n	P _n /1.2	φ _c P _n	P _n /1.2	φ _c P _n
	ASD	LFRD	ASD	LFRD	ASD	LFRD	ASD	LFRD	ASD	LFRD	ASD	LFRD
0	980	1420	977	1320	718	1170	680	1020	580	850	520	780
6	934	1400	931	1290	722	1110	643	980	563	840	500	752
7	917	1380	915	1270	722	1090	630	946	552	820	490	737
8	897	1350	897	1250	706	1060	615	925	539	810	478	718
9	875	1310	877	1170	680	1030	599	900	525	789	468	700
10	850	1270	852	1110	644	1000	580	875	510	765	458	683
11	825	1240	822	1100	644	972	572	848	493	741	437	657
12	798	1200	797	1050	605	940	553	816	475	714	427	633
14	739	1110	754	963	578	868	501	733	438	658	380	583
15	700	1050	706	911	553	811	479	720	419	629	370	556

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Example 4-12: Try a W12 shape for a strength > 840 k

Assume that the strong direction controls and enter Table 4-1a for F_y = 50 ksi and L_c = 12 ft.

Table 4-1a (continued)
Available Strength in Axial Compression, kips
W-Shapes
F_y = 50 ksi

Shape lb/ft	W12											
	96		87		79		72		65		55	
Design	P _n /1.2	φ _c P _n	P _n /1.2	φ _c P _n	P _n /1.2	φ _c P _n	P _n /1.2	φ _c P _n	P _n /1.2	φ _c P _n	P _n /1.2	φ _c P _n
	ASD	LFRD	ASD	LFRD	ASD	LFRD	ASD	LFRD	ASD	LFRD	ASD	LFRD
0	844	1270	766	1150	695	1040	632	949	572	859	549	825
6	811	1220	736	1110	667	1000	606	911	549	825	549	825
7	800	1200	726	1090	657	988	597	898	540	812	540	812
8	787	1180	714	1070	646	971	587	883	531	798	531	798
9	772	1160	700	1050	634	953	576	866	521	783	521	783
10	756	1140	686	1030	623	937	565	849	510	769	510	769
11	738	1110	670	1010	610	917	554	832	497	747	497	747
12	720	1080	653	981	591	897	539	806	484	728	484	728
14	680	1020	616	925	556	836	505	759	456	685	456	685
15	628	950	566	866	526	800	480	725	441	653	441	653

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Example 4-12: From Table 1-1 (1-28) a W12 x 79 has:

$$r_x = 5.34in \quad r_y = 3.05in \quad A_g = 23.2in^2$$

$$r_x/r_y = \frac{5.34in}{3.05in} = 1.75$$

Table 4-1a (continued)
Available Strength in Axial Compression, kips
W-Shapes
F_y = 50 ksi

Shape lb/ft	W12											
	96		87		79		72		65		55	
Design	P _n /1.2	φ _c P _n	P _n /1.2	φ _c P _n	P _n /1.2	φ _c P _n	P _n /1.2	φ _c P _n	P _n /1.2	φ _c P _n	P _n /1.2	φ _c P _n
	ASD	LFRD	ASD	LFRD	ASD	LFRD	ASD	LFRD	ASD	LFRD	ASD	LFRD
0	844	1270	766	1150	695	1040	632	949	572	859	549	825
6	811	1220	736	1110	667	1000	606	911	549	825	549	825
7	800	1200	726	1090	657	988	597	898	540	812	540	812
8	787	1180	714	1070	646	971	587	883	531	798	531	798
9	772	1160	700	1050	634	953	576	866	521	783	521	783
10	756	1140	686	1030	623	937	565	849	510	769	510	769
11	738	1110	670	1010	610	917	554	832	497	747	497	747
12	720	1080	653	981	591	897	539	806	484	728	484	728
14	680	1020	616	925	556	836	505	759	456	685	456	685
15	628	950	566	866	526	800	480	725	441	653	441	653

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Example 4-12: From Table 1-1 (1-28) a W12 x 79 has:

$$\text{Actual } \frac{L_{cx}}{r_x/r_y} = \frac{20ft}{1.75} = 11.43ft < 12ft$$

Table 4-1a (continued)
Available Strength in Axial Compression, kips
W-Shapes
F_y = 50 ksi

Shape lb/ft	W12											
	96		87		79		72		65		55	
Design	P _n /1.2	φ _c P _n	P _n /1.2	φ _c P _n	P _n /1.2	φ _c P _n	P _n /1.2	φ _c P _n	P _n /1.2	φ _c P _n	P _n /1.2	φ _c P _n
	ASD	LFRD	ASD	LFRD	ASD	LFRD	ASD	LFRD	ASD	LFRD	ASD	LFRD
0	844	1270	766	1150	695	1040	632	949	572	859	549	825
6	811	1220	736	1110	667	1000	606	911	549	825	549	825
7	800	1200	726	1090	657	988	597	898	540	812	540	812
8	787	1180	714	1070	646	971	587	883	531	798	531	798
9	772	1160	700	1050	634	953	576	866	521	783	521	783
10	756	1140	686	1030	623	937	565	849	510	769	510	769
11	738	1110	670	1010	610	917	554	832	497	747	497	747
12	720	1080	653	981	591	897	539	806	484	728	484	728
14	680	1020	616	925	556	836	505	759	456	685	456	685
15	628	950	566	866	526	800	480	725	441	653	441	653

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Example 4-12: By interpolation, a W12 x 79 has:

$$910 + (887 - 910)0.43 = 900.1k$$

$$\phi_c P_n = 900.1k > 840k \quad \text{OK}$$

Table 4-1a (continued)
Available Strength in Axial Compression, kips
W-Shapes
F_y = 50 ksi

Shape lb/ft	W12											
	96		87		79		72		65		55	
Design	P _n /1.2	φ _c P _n	P _n /1.2	φ _c P _n	P _n /1.2	φ _c P _n	P _n /1.2	φ _c P _n	P _n /1.2	φ _c P _n	P _n /1.2	φ _c P _n
	ASD	LFRD	ASD	LFRD	ASD	LFRD	ASD	LFRD	ASD	LFRD	ASD	LFRD
0	844	1270	766	1150	695	1040	632	949	572	859	549	825
6	811	1220	736	1110	667	1000	606	911	549	825	549	825
7	800	1200	726	1090	657	988	597	898	540	812	540	812
8	787	1180	714	1070	646	971	587	883	531	798	531	798
9	772	1160	700	1050	634	953	576	866	521	783	521	783
10	756	1140	686	1030	623	937	565	849	510	769	510	769
11	738	1110	670	1010	610	917	554	832	497	747	497	747
12	720	1080	653	981	591	897	539	806	484	728	484	728
14	680	1020	616	925	556	836	505	759	456	685	456	685
15	628	950	566	866	526	800	480	725	441	653	441	653

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Example 4-12: Try a W14 shape for a strength > 840 k

Assume that the strong direction controls and enter Table 4-1a for F_y = 50 ksi and L_c = 12 ft.

Table 4-1a (continued)
Available Strength in Axial Compression, kips
W-Shapes
F_y = 50 ksi

Shape lb/ft	W14											
	82		74		68		61		53		48	
Design	P _n /1.2	φ _c P _n	P _n /1.2	φ _c P _n	P _n /1.2	φ _c P _n	P _n /1.2	φ _c P _n	P _n /1.2	φ _c P _n	P _n /1.2	φ _c P _n
	ASD	LFRD	ASD	LFRD	ASD	LFRD	ASD	LFRD	ASD	LFRD	ASD	LFRD
0	719	1080	653	981	599	900	536	805	467	702	422	624
6	676	1020	614	922	562	845	503	756	421	653	380	572
7	661	993	600	892	550	826	482	739	406	640	365	557
8	644	966	585	879	536	805	470	720	389	625	351	527
9	625	940	568	864	520	785	459	699	371	617	342	509
10	605	910	550	827	503	756	450	678	351	628	318	471
11	585	880	532	800	485	726	440	656	331	604	298	442
12	565	844	514	767	466	701	418	630	310	578	278	411
14	514	772	462	701	426	640	391	597	292	541	260	378
15	484	722	444	671	405	615	381	544	280	521	249	359

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➤ Example 4-12: From Table 1-1 (1-28), the **W14 x 82**:

$$r_x = 6.05 \text{ in} \quad r_y = 2.48 \text{ in} \quad r_x/r_y = \frac{6.05 \text{ in}}{2.48 \text{ in}} = 2.44$$

$$\text{Actual } \frac{L_{cx}}{r_x/r_y} = \frac{20 \text{ ft}}{2.44} = 8.198 \text{ ft} < 12 \text{ ft} \quad \text{Assume } L_c = 9 \text{ ft.}$$

Table 4-1a (continued)
Available Strength in Axial Compression, kips
W-Shapes
 $F_y = 50 \text{ ksi}$

$\phi_c P_n = 854 \text{ k}$

Shape	W14-c														
	82		74		68		61		53		48		43#1		
lb/ft	82	74	68	61	53	48	43#1	82	74	68	61	53	48	43#1	
Design	$\phi_c P_n$	$\phi_c P_n$	$\phi_c P_n$	$\phi_c P_n$	$\phi_c P_n$	$\phi_c P_n$	$\phi_c P_n$	$\phi_c P_n$	$\phi_c P_n$	$\phi_c P_n$	$\phi_c P_n$	$\phi_c P_n$	$\phi_c P_n$	$\phi_c P_n$	
Least radius of gyration, r_y	0	719	1080	653	981	599	900	536	805	467	702	422	634	374	562
	6	579	1020	614	922	562	846	503	736	423	633	380	572	339	510
9	606	940	568	854	507	782	465	699	371	557	334	502	297	447	
	11	584	878	531	797	485	729	433	651	331	497	298	447	264	397
12	562	844	510	767	466	701	416	626	318	465	279	419	247	371	
13	538	809	480	735	446	671	388	599	298	433	259	389	229	345	
14	514	772	447	701	426	640	360	571	287	401	240	360	212	318	
15	460	725	414	667	405	606	341	543	269	371	212	334	192	292	

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➤ Example 4-12: From Table 1-1 (1-28), the **W14 x 74**:

$$r_x = 6.04 \text{ in} \quad r_y = 2.48 \text{ in} \quad r_x/r_y = \frac{6.04 \text{ in}}{2.48 \text{ in}} = 2.44$$

$$\text{Actual } \frac{L_{cx}}{r_x/r_y} = \frac{20 \text{ ft}}{2.44} = 8.198 \text{ ft} < 12 \text{ ft} \quad \text{Assume } L_c = 9 \text{ ft.}$$

Table 4-1a (continued)
Available Strength in Axial Compression, kips
W-Shapes
 $F_y = 50 \text{ ksi}$

$\phi_c P_n = 854 \text{ k}$

Shape	W14-c														
	82		74		68		61		53		48		43#1		
lb/ft	82	74	68	61	53	48	43#1	82	74	68	61	53	48	43#1	
Design	$\phi_c P_n$	$\phi_c P_n$	$\phi_c P_n$	$\phi_c P_n$	$\phi_c P_n$	$\phi_c P_n$	$\phi_c P_n$	$\phi_c P_n$	$\phi_c P_n$	$\phi_c P_n$	$\phi_c P_n$	$\phi_c P_n$	$\phi_c P_n$	$\phi_c P_n$	
Least radius of gyration, r_y	0	719	1080	653	981	599	900	536	805	467	702	422	634	374	562
	6	579	1020	614	922	562	846	503	736	423	633	380	572	339	510
9	606	940	568	854	507	782	465	699	371	557	334	502	297	447	
	11	584	878	531	797	485	729	433	651	331	497	298	447	264	397
12	562	844	510	767	466	701	416	626	318	465	279	419	247	371	
13	538	809	480	735	446	671	388	599	298	433	259	389	229	345	
14	514	772	447	701	426	640	360	571	287	401	240	360	212	318	
15	460	725	414	667	405	606	341	543	269	371	212	334	192	292	

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➤ Example 4-12: By interpolation, a **W14 x 74** has

$$\phi_c P_n = 874 \text{ k} > 840 \text{ k} \quad \text{OK}$$

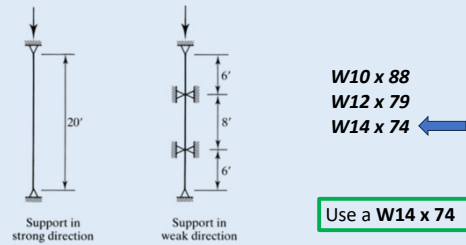
Table 4-1a (continued)
Available Strength in Axial Compression, kips
W-Shapes
 $F_y = 50 \text{ ksi}$

Shape	W14-c														
	82		74		68		61		53		48		43#1		
lb/ft	82	74	68	61	53	48	43#1	82	74	68	61	53	48	43#1	
Design	$\phi_c P_n$	$\phi_c P_n$	$\phi_c P_n$	$\phi_c P_n$	$\phi_c P_n$	$\phi_c P_n$	$\phi_c P_n$	$\phi_c P_n$	$\phi_c P_n$	$\phi_c P_n$	$\phi_c P_n$	$\phi_c P_n$	$\phi_c P_n$	$\phi_c P_n$	
Least radius of gyration, r_y	0	719	1080	653	981	599	900	536	805	467	702	422	634	374	562
	6	579	1020	614	922	562	846	503	736	423	633	380	572	339	510
8	644	960	585	879	530	805	479	720	389	585	351	527	312	470	
	9	606	940	568	854	507	782	465	699	371	557	334	502	297	447
11	584	878	531	797	485	729	433	651	331	497	298	447	264	397	
12	562	844	510	767	466	701	416	626	318	465	279	419	247	371	
13	538	809	480	735	446	671	388	599	298	433	259	389	229	345	
14	514	772	447	701	426	640	360	571	287	401	240	360	212	318	
15	460	725	414	667	405	606	341	543	269	371	212	334	192	292	

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➤ Example 4-12: The column shown below is subjected to a service dead load of 140 k and a service live load of 420 k. Use **A992 steel** ($F_y = 50 \text{ ksi}$) and select a **W-shape**.



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Let's work on some problems



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Any questions?



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