

Reinforced Concrete Beam Example #1

Let's use the failure models to predict the ultimate strength-to-weight (SWR) of one of our reinforced concrete beams from lab

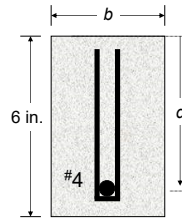
Consider a beam with the following characteristics:

- Concrete strength $f'_c = 4,000$ psi
- Steel strength $f_y = 60,000$ psi
- The tension reinforcement will be 1 #4 rebar
- The shear reinforcement will be #3 rebar, U-shaped, 3 in. spacing
- Use minimum cover of 0.75 in. and a width to accommodate the reinforcement

1

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Reinforcing bars are denoted by the bar number. The diameter and area of standard rebars are shown below.



Bar #	Diameter (in.)	As (in. ²)
3	0.375	0.11
4	0.500	0.20
5	0.625	0.31
6	0.750	0.44
7	0.875	0.60
8	1.000	0.79
9	1.128	1.00
10	1.270	1.27
11	1.410	1.56

2

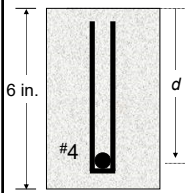
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Based on the choice of reinforcement we can compute an estimate of b and d

$$b \geq 2(\text{Minimum cover}) + 2(\text{\#3 rebar diameter}) + 1(\text{\#4 rebar diameter})$$

$$b \geq 2(0.75 \text{ in}) + 2(0.375 \text{ in}) + 1(0.5 \text{ in})$$

$$= 2.75 \text{ in.}$$



3

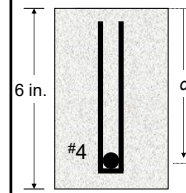
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If we allow a minimum cover under the rebar we can estimate d

$$d = 6 - \text{Minimum cover} - \text{\#3 rebar diameter} - \frac{\text{Half of \#4 bar diameter}}{2}$$

$$d = 6 - 0.75 - 0.375 - \frac{0.5}{2}$$

$$d = 4.625 \text{ in.}$$



4

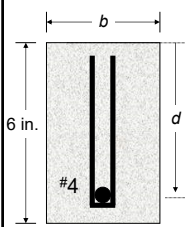
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We now have values for b , d , and A_s

$$M = A_s f_y \left(d - 0.59 \frac{A_s f_y}{f'_c b} \right)$$

The A_s for one #4 rebar is:

$$A_s = 0.20 \text{ in.}^2$$



5

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Compute the moment capacity

$$M = A_s f_y \left(d - 0.59 \frac{A_s f_y}{f'_c b} \right)$$

$$= 0.20 \text{ in.}^2 (60 \text{ ksi}) \left(4.625 \text{ in.} - 0.59 \frac{0.20 \text{ in.}^2 (60 \text{ ksi})}{4 \text{ ksi} (2.75 \text{ in.})} \right)$$

$$= 47.78 \text{ k} \cdot \text{in} \Rightarrow P_{tension} = \frac{M}{4} = 11.94 \text{ kips}$$

6

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Let's check the shear model

$$P_{shear} = 2 \left(\frac{A_v f_y d}{s} + 2 \sqrt{f'_c} b d \right)$$

Area of a #3 rebar

$$= 2 \left(\frac{2(0.11 \text{ in.}^2)(60,000 \text{ psi})4.625 \text{ in.}}{3 \text{ in.}} + 2 \sqrt{4,000 \text{ psi}} (2.75 \text{ in.})(4.625 \text{ in.}) \right)$$

Shear reinforcement spacing

$$= 43,917 \text{ lb.} = \boxed{43.9 \text{ kips}}$$

Since $P_{tension} < P_{shear}$ therefore $P_{tension}$ controls

7

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Let's check the reinforcement ratio

$$\rho = \frac{A_s}{bd}$$

$$\rho = 0.85 \beta_1 \frac{c f'_c}{d f_y}$$

ρ as function of c/d

To compute ρ , first we need to estimate β_1

8

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The height of the stress box, a , is defined as a percentage of the depth to the neural axis

$$f'_c \leq 4000 \text{ psi} \Rightarrow \beta_1 = 0.85$$

$$f'_c \geq 4000 \text{ psi}$$

$$\beta_1 = 0.85 - 0.05 \left(\frac{f'_c - 4000}{1000} \right) \geq 0.65$$

$$\beta_1 = 0.85 - 0.05 \left(\frac{4,000 - 4,000}{1,000} \right) = \boxed{0.85}$$

9

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Check the reinforcement ratio for the maximum steel allowed

$$\rho = \frac{A_s}{bd} = \frac{0.20 \text{ in.}^2}{2.75 \text{ in.}(4.625 \text{ in.})} = 0.0157$$

$$\rho_{tension} = 0.85 \beta_1 \frac{c f'_c}{d f_y} = 0.85(0.85)0.375 \frac{4 \text{ ksi}}{60 \text{ ksi}}$$

$$= 0.0181$$

$$\rho < \rho_{tension}$$

10

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The minimum force controls

$$P_{tension} = 11.94 \text{ kips}$$

$$P_{shear} = 43.92 \text{ kips}$$

$$P = \boxed{11.94 \text{ kips}}$$

11

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An estimate of the weight of the beam can be made as:

$$W = \frac{(2.75 \text{ in.})(6 \text{ in.})(30 \text{ in.})}{1728 \text{ in.}^3/\text{ft.}^3} \left(\frac{145 \text{ lb.}}{\text{ft.}^3} \right)$$

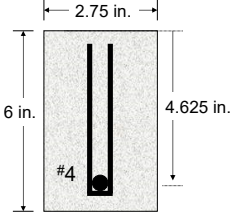
$$+ \frac{(0.20 \text{ in.}^2)(30 \text{ in.})}{1728 \text{ in.}^3/\text{ft.}^3} \left(\frac{490 \text{ lb.} - 145 \text{ lb.}}{\text{ft.}^3} \right)$$

$$= 41.54 \text{ lb.} + 1.20 \text{ lb.} = \boxed{42.74 \text{ lb.}}$$

12

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In summary, this reinforced concrete beam will fail in tension



$\Rightarrow P = 11.94 \text{ kips}$

$SWR = \frac{11,907 \text{ lb}}{42.74 \text{ lb}} = 279$

This beam should fail in tension

13

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The cost of steel may be estimated as follows:

$$\text{Cost of steel} = \frac{A_s L}{1,728 \frac{\text{in.}^3}{\text{ft.}^3}} \left(490 \frac{\text{lb.}}{\text{ft.}^3} \right) \left(\frac{\$530}{\text{ton}} \right) \left(\frac{\text{ton}}{2,000 \text{ lb.}} \right)$$

where A_s is the cross-sectional area of steel rebars, L is the length of the steel rebars, and 490 lb./ft.^3 is the unit weight of steel.

14

Reinforced Concrete Beam Example #1

For example, if one #4 rebar is placed in the beam the steel cost is estimated as:

Bar #	Diameter (in.)	As (in. ²)
3	0.375	0.11
4	0.500	0.20
5	0.625	0.31
6	0.750	0.44
7	0.875	0.60
8	1.000	0.79
9	1.128	1.00
10	1.270	1.27
11	1.410	1.56

$$\text{Cost of steel} = \frac{(0.2 \text{ in.}^2)(30 \text{ in.})}{1,728 \frac{\text{in.}^3}{\text{ft.}^3}} \left(490 \frac{\text{lb.}}{\text{ft.}^3} \right) \left(\frac{\$530}{\text{ton}} \right) \left(\frac{\text{ton}}{2,000 \text{ lb.}} \right)$$

= \$0.45

15

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Consider the following mix for a yd.³ of concrete developed using the ACI mix design procedure.

Component	Amount (lb)
Water	315
Cement	553
Coarse aggregate	1,641
Fine aggregate	1,431

16

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The cost of the concrete required for a 2.75 in. by 6 in. by 30 in. beam is estimated as:

$$\text{Cost of cement} = \frac{2.75 \text{ in.}(6 \text{ in.})30 \text{ in.}}{1,728 \frac{\text{in.}^3}{\text{ft.}^3}} \left(\frac{553 \text{ lb.}}{27 \text{ ft.}^3} \right) \left(\frac{\$130}{\text{ton}} \right) \left(\frac{\text{ton}}{2,000 \text{ lb.}} \right)$$

= \$0.38

$$\text{Cost of coarse aggregate} = \frac{2.75 \text{ in.}(6 \text{ in.})30 \text{ in.}}{1,728 \frac{\text{in.}^3}{\text{ft.}^3}} \left(\frac{1,641 \text{ lb.}}{27 \text{ ft.}^3} \right) \left(\frac{\$18}{\text{ton}} \right) \left(\frac{\text{ton}}{2,000 \text{ lb.}} \right)$$

= \$0.16

17

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The cost of the concrete required for a 2.75 in. by 6 in. by 30 in. beam is estimated as:

$$\text{Cost of fine aggregate} = \frac{2.75 \text{ in.}(6 \text{ in.})30 \text{ in.}}{1,728 \frac{\text{in.}^3}{\text{ft.}^3}} \left(\frac{1,431 \text{ lb.}}{27 \text{ ft.}^3} \right) \left(\frac{\$10}{\text{ton}} \right) \left(\frac{\text{ton}}{2,000 \text{ lb.}} \right)$$

= \$0.08

The cost concrete is estimated as: \$0.62

The cost of the reinforced concrete beam is estimated as: \$1.07

18

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The cost adjustment for the reinforced concrete beam is:

If cost < \$1.50 then: $Cost\ Factor = 1$

If cost > \$1.50 then:

$$Cost\ Factor = \frac{\$1.50}{Cost}$$

19

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If the unadjusted **SWR** for a beam is 279 and the cost is \$1.07, then the cost adjusted **SWR** is:

$$SWR_{Adjusted} = SWR \times Cost\ Factor$$

$$SWR_{Adjusted} = 279 \times 1 = 279$$

20

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Questions?



21